



Massachusetts Gaming Commission
101 Federal Street, 12th Floor
Boston, MA 02110

**2016 COMMUNITY MITIGATION FUND
RESERVE / SPECIFIC IMPACT APPLICATION**

CHECK BOX IF REQUESTING THE CREATION OF A
MITIGATION RESERVE FUND FOR A COMMUNITY

X

APPLICATIONS DUE NO LATER THAN FEBRUARY 1, 2016.

For anyone with specific impacts, please complete the gray boxes 1-4 beginning on page 2. If you are not applying for mitigation of specific impacts by February 1, 2016, you do not need to complete grayed boxes 1-4.

1.	NAME OF MUNICIPALITY/GOVERNMENT ENTITY/DISTRICT
2.	DEPARTMENT RECEIVING FUNDS
3.	NAME AND TITLE OF INDIVIDUAL RESPONSIBLE FOR HANDLING OF FUNDS
4.	ADDRESS OF INDIVIDUAL RESPONSIBLE FOR HANDLING OF FUNDS
5.	PHONE # AND EMAIL ADDRESS OF INDIVIDUAL RESPONSIBLE FOR HANDLING OF FUNDS
6.	NAME AND TITLE OF INDIVIDUAL AUTHORIZED TO COMMIT FUNDS ON BEHALF OF MUNICIPALITY/GOVERNMENTAL ENTITY
7.	ADDRESS OF INDIVIDUAL AUTHORIZED TO COMMIT FUNDS ON BEHALF OF MUNICIPALITY/GOVERNMENTAL ENTITY
8.	PHONE # AND EMAIL ADDRESS OF INDIVIDUAL AUTHORIZED TO COMMIT FUNDS ON BEHALF OF MUNICIPALITY/GOVERNMENTAL ENTITY
9.	NAME OF GAMING LICENSEE

2016 RESERVE / SPECIFIC COMMUNITY MITIGATION IMPACT APPLICATION
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1. IMPACT DESCRIPTION

Please describe in detail the impact that is attributed to the construction of a gaming facility. Please provide support for the determination that the construction of the gaming facility caused or is causing the impact.

The City of Lynn is an older urban City with a large blue collar population. It is anticipated that many of the construction workers employed by the Licensee will reside in Lynn. These individuals will be traveling on Lynn's congested roadways during rush hour further congesting Lynn roads. It is likely the majority of these individuals will utilize Route 1A and Route 107 to travel back and forth to the construction site in Everett. Concrete/cement contractors from Lynn and the immediate surrounding areas will likely be utilized during the construction phase. These construction vehicles and trucks would be utilizing Routes 1A and 107 to and from the Everett facility.

Routes 1A and 107 are heavily traveled thoroughfares and during construction will be further congested. The General Electric facility is located between Routes 1A and 107 and sees heavy traffic during the rush hour commute. The construction of the Everett facility will further exacerbate traffic issues in the City of Lynn.

The City of Lynn is an older urban Gateway City with a population of over 90,000. Lynn is the largest city on the North Shore and tenth largest city in Massachusetts. Lynn's recently completed Waterfront Master Plan contains waterfront parcels that have been identified by the Commonwealth of Massachusetts as priority development sites. These parcels are a mere 8 miles from the proposed facility. In addition, thousands of Lynn, Swampscott, Marblehead and Salem residents commute to Boston and beyond each morning utilizing Lynn roads that lead them directly to the proposed facility.

Although the City of Lynn has steadily been working on reinventing itself, attracting loft and condo owners, one issue remains. The City is plagued by a lack of direct, free flowing traffic into and out of the city limits. Somewhere along each entry point, motorists must pass through residential neighborhoods or navigate limited access roads with traffic signals and numerous stops. Unlike urban communities abutting major thoroughfares such as Routes 128, 495 and 1, the City of Lynn's commercial base and resulting economics are limited to what can be carried over these aging local roads. In fact, during a recent study administered by economist Dr. Barry Bluestone of Northeastern University, the lack of free flowing vehicular transportation was identified as one of the major "deal

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breakers" when it came to attracting new business to Lynn.

In an attempt to remedy these issues, the City has been tirelessly lobbying for funds to design, engineer, and implement a variety of improvements and alternatives aimed at streamlining vehicular and transit oriented commuters and commerce into and out of the City's core business districts.

To date, Lynn has been successful lobbying for major improvements to one of the City's core business districts along Route 129. Major roadway improvements were undertaken and completed by the Commonwealth of Massachusetts. Lynn also obtained funding to offer a ferry service in and out of Boston for the past two summers. Unfortunately, no funding has yet to be secured for the continuation of the ferry service in the summer of 2016. The ferry service eliminates significant vehicular traffic during the morning and evening rush hour commutes. The City is actively seeking funding for additional stops in Lynn on our commuter rail line, and exploring ways to create additional access to our waterfront, which is now primed for development. In addition, we are in the permitting phases of a variety of city intersections including access to our Waterfront off Route 1A, streamlining traffic on Route 107 and the extension of the Blue Line.

Across the Commonwealth, large municipal projects such as Quincy's Marina Bay and the new Quincy Center/Street Works Initiative, Somerville's Assembly Square/Inner Belt and Worcester's City Square are reinvigorating the economies of older Urban Centers. Governor Baker and his Chief Economic Aid Jay Ash have observed Lynn's potential for significant growth and committed to work with Lynn to assist us to reach our goals. Governor Baker and Secretary Ash believe that Lynn has positioned itself for redevelopment of both our Waterfront and Downtown. As the economy improves, much like these other cities, State and local officials believe that Lynn is poised to continue its economic transformation and break through as one of the areas main, cultural, economic and tourist hubs.

However, in attempting to bring in businesses to the City, many corporate leaders are looking to areas closer to the Everett facility. For more than a decade Lynn officials have attempted to attract a hotel to the City of Lynn. To date, this endeavor has been unsuccessful. While Lynn is only 8 miles from the Everett facility, hotels and other businesses have selected locations closer to the Everett facility with the anticipation of a fertile market. The significant traffic congestion in Lynn on a daily basis makes the 8 mile commute appear much longer in the eyes of businesses that we have sought to locate in Lynn. The

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increased traffic on Lynn's roadways during the course of the construction will only worsen the perceived traffic concerns of prospective businesses.

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2. PROPOSED MITIGATION

Please identify below the manner in which the funds are proposed to be used. Please provide documentation (e.g. - invoices, proposals, estimates, etc.) adequate for the Commission to ensure that the funds will be used for the cost of mitigating the impact from the construction of a proposed gaming establishment. Please describe how the mitigation request will address the specific impact indicated. Please attach additional sheets/supplemental materials if necessary.

Lynn would utilize these funds to alleviate the significant traffic issues facing our City. Lynn has been actively seeking funding to create a left hand turn into the ferry entrance for commuters traveling south on Route 1A. At the present time, ferry travelers are forced to drive past the ferry entrance and take a legal U-turn more than a half mile away from the ferry terminal.

The City is also actively seeking funding to continue to operate the ferry this coming summer. The ferry shuttle from Lynn to Boston has been operational the past two summers with the assistance of state funds. At this time, no such funding has been allotted to the City of Lynn. Just last week, the City hosted the owner of the largest ferry service in the world to look at ways to continue to operate and to expand the ferry service. The City has attached funding estimates to achieve this much needed goal.

The City has conducted numerous traffic studies to synchronize the traffic lights on Routes 107 to alleviate congestion. Route 107 has been identified as a priority by the Commonwealth of Massachusetts because of traffic congestion. In fact, the Commonwealth of Massachusetts is holding a series of public hearings to discuss ways that Route 107 could be improved. Unfortunately, not all of Route 107 is under the jurisdiction of the Commonwealth. More than three miles of Route 107 including seven major intersections are under the jurisdiction of the City. Improvements undertaken by the Commonwealth of Massachusetts on sections of the road under its control will not have a maximum benefit unless the City secures funding to upgrade traffic signals under the City's control. Traffic issues along the Route 107 corridor will only worsen when a new Market Basket is constructed and opened in 2016.

The City is also seeking funding to add additional stops on the Rockport/Newburyport Commuter Rail Line. Recently, a developer purchased a 65 acre site adjacent to the Riverworks commuter rail stop. However, presently, only employees of the General Electric can utilize this stop. It is envisioned that the property will be developed into several hundred residential units directly overlooking Boston Harbor. While the City strongly supports this

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proposed project, the City recognizes that the project will increase the significant traffic issues that exist at this time. In order for this project to proceed, significant upgrades to the Riverwork station must be completed.

If awarded community mitigation funding, the City will consult with State officials including Secretary Ash and representatives of Massachusetts Highway to determine the best manner to expend these funds to lessen traffic and maximize the economic development in Lynn

3. IMPACT CONTROLS/ADMINISTRATION OF IMPACT FUNDS

Please provide detail regarding the controls that will be used to ensure that funds will only be used to address the specific impact. If non-governmental entities will receive any funds, please describe what reporting will be required and how the applicant will remedy any misuse of funds.

The Chief Financial Officer of the City of Lynn has insisted that any community mitigation funding awarded to the City of Lynn be placed in a separate revolving account. In this manner, the funding would only be permitted if it addressed the specific transportation needs of the City. Community Development Director Marsh would be required to obtain the approval of both the Mayor and the City Council prior to any expenditure of community mitigation funding.

City officials have worked tirelessly to secure funding for the projects identified in Section 2 of the Application. The City is hopeful that a public/private partnership can result to bring these projects to completion. However, the City understands that Lynn will need to contribute funding to any other sources of public and private financing that becomes available. The community mitigation monies would ensure that Lynn could meet its obligations to bring the traffic improvements to fruition.

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4. RELEVANT EXCERPTS FROM HOST OR SURROUNDING COMMUNITY AGREEMENTS

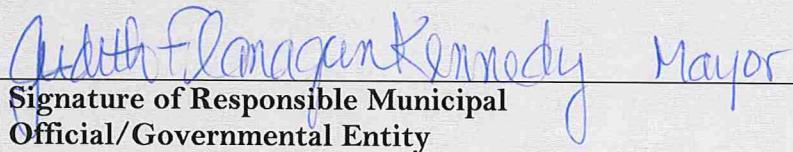
Please describe and include excerpts from any relevant sections of any Host or Surrounding Community Agreement. Please explain how this impact was either anticipated or not anticipated in that Agreement.

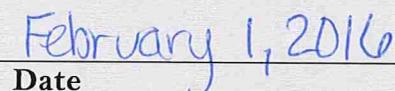
Attached please find a Neighboring Community Agreement between the City of Lynn and Wynn MA, LLC.

Since the execution of this Agreement, Wynn MA, LLC has selected Suffolk Construction as the general contractor to construct its facility. Suffolk Construction employees a significant number of employees in the Greater Lynn area. Traditionally, Suffolk has hired many Lynn tradesmen as subcontractors and vendors on local construction projects. The selection of Suffolk Construction ensures that Lynn will be adversely effected as a result of the injection of increased vehicular traffic.

CERTIFICATION BY MUNICIPALITY/GOVERNMENTAL ENTITY

On behalf of the aforementioned municipality/governmental entity I hereby certify that the funds that are requested in this application will be used solely for the purposes articulated in this Application.


Signature of Responsible Municipal Official/Governmental Entity


Date

APPROVAL OF THE MASSACHUSETTS GAMING COMMISSION

On behalf of the Massachusetts Gaming Commission, the Commission hereby authorizes the payment from the Community Mitigation Fund in accordance with M.G.L. c. 23K as outlined in this Application.

Executive Director

Date

Ombudsman

Date

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**Massachusetts Gaming Commission
101 Federal Street, 12th Floor
Boston, MA 02110**

**INSTRUCTIONS FOR FILING FOR THE
2016 COMMUNITY MITIGATION FUND RESERVE / SPECIFIC IMPACT
APPLICATION OR 2016 TRANSPORTATION PLANNING GRANT APPLICATION**

1. 2015 Mitigation Reserve Fund - continuation

Note: Communities that received approval of the 2015 Mitigation Reserve Fund request **do not** need to submit any application to preserve its reserve in 2016.

2. 2016 Mitigation Reserve Fund Only

If a Community is filing for a **mitigation reserve fund only**, please use the application entitled "**2016 COMMUNITY MITIGATION FUND RESERVE / SPECIFIC IMPACT APPLICATION**", **check the box on page 1** and fill out all the boxes in blue. **Skip grayed boxes 1-4**. Detail regarding the use of the reserve fund can be provided to the Commission on a rolling basis upon consultation with the Commission.

3. 2016 Community Mitigation Fund Specific Impact Application

If a Community or other Applicant has a **specific impact**, please use the application entitled "**2016 COMMUNITY MITIGATION FUND RESERVE / SPECIFIC IMPACT APPLICATION**" and **fill out entire application form**.

4. 2016 Community Mitigation Reserve Fund and 2016 Community Mitigation Fund Specific Impact Application

If a Community or other Applicant is requesting **both a 2016 Mitigation Reserve Fund and has a specific impact**, please **fill out the entire application form** entitled "**2016 COMMUNITY MITIGATION FUND RESERVE / SPECIFIC IMPACT APPLICATION**".

5. 2016 Transportation Planning Grant Application

If a Community is filing for a **2016 Transportation Planning Grant**, please use the application entitled "**2016 TRANSPORTATION PLANNING GRANT APPLICATION**". Please **fill out the entire application form**.

**Any questions contact: John S. Ziembra, Ombudsman
101 Federal Street, 12th Floor, Boston, MA 02110
(617) 979-8423 or john.s.ziembra@state.ma.us**

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An application must be received by February 1, 2016 to meet the application deadline.

Applications should be sent to: www.commbuys.com. Applicants that are not part of the COMMBUYS system should contact Mary Thurlow of the Commission's Ombudsman's Office well in advance of the February 1, 2016 deadline to make arrangements for submission of the application by the deadline. Mary Thurlow can be contacted at 617-979-8420 or at mary.thurlow@state.ma.us.

If you have any questions regarding COMMBUYS, please contact COMMBUYS Help Desk assistance at 1-888-627-8283 or COMMBUYS@state.ma.us.

Attachments

- Traffic Volume Count Data
- 2014 Lynn to Boston Ferry Schedule
- Lynn Commuter Ferry Ridership Data
- Intersection Capacity Analyses
- Preliminary Cost Estimates
- Signal Warrant Worksheet

Traffic Volume Count Data



PRECISION DATA INDUSTRIES, LLC
Office: 508.481.3999 Fax: 508.545.1234
Email: datarequests@pdillc.com

Traffic Counts with Precision



Imagery Date: 8/24/2013 42°27'26.62" N 70°57'04.27" W elev 11 ft eye alt 1808 ft

Client:
VHB

Engineer:
K. Keen

Site Code:
11942.03

Date:
Tuesday 11/18/14

PDI Job Number:
144179

City, State:
Lynn, MA



PRECISION
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P.O. Box 301 Berlin, MA 01503
Office: 508.481.3999 Fax: 508.545.1234
Email: datarequests@pdillc.com

N/S: Shepard Street/ Marine Boulevard
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 A
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Cars - Heavy Vehicles

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
07:00 AM	4	0	7	0	14	640	1	2	2	7	1	0	6	199	3	0	886
07:15 AM	5	0	6	0	12	621	2	5	2	1	1	0	3	227	9	0	894
07:30 AM	6	0	5	0	12	579	0	7	3	1	2	0	4	290	12	1	922
07:45 AM	5	0	6	0	14	554	3	12	2	0	4	0	4	310	11	2	927
Total	20	0	24	0	52	2394	6	26	9	9	8	0	17	1026	35	3	3629
08:00 AM	3	0	12	0	13	545	5	15	4	0	1	0	6	236	10	0	850
08:15 AM	2	0	8	0	12	549	4	7	3	3	2	0	3	287	11	0	891
08:30 AM	4	0	9	0	13	444	3	10	3	0	2	0	4	266	13	0	771
08:45 AM	2	0	6	0	11	457	2	10	4	0	0	0	2	287	9	0	790
Total	11	0	35	0	49	1995	14	42	14	3	5	0	15	1076	43	0	3302
Grand Total	31	0	59	0	101	4389	20	68	23	12	13	0	32	2102	78	3	6931
Apprch %	34.4	0	65.6	0	2.2	95.9	0.4	1.5	47.9	25	27.1	0	1.4	94.9	3.5	0.1	
Total %	0.4	0	0.9	0	1.5	63.3	0.3	1	0.3	0.2	0.2	0	0.5	30.3	1.1	0	
Cars	25	0	50	0	94	4292	13	66	14	7	7	0	16	1993	71	3	6651
% Cars	80.6	0	84.7	0	93.1	97.8	65	97.1	60.9	58.3	53.8	0	50	94.8	91	100	96
Heavy Vehicles	6	0	9	0	7	97	7	2	9	5	6	0	16	109	7	0	280
% Heavy Vehicles	19.4	0	15.3	0	6.9	2.2	35	2.9	39.1	41.7	46.2	0	50	5.2	9	0	4

	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					Int. Total
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	
Start Time																					
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:00 AM																					
07:00 AM	4	0	7	0	11	14	640	1	2	657	2	7	1	0	10	6	199	3	0	208	886
07:15 AM	5	0	6	0	11	12	621	2	5	640	2	1	1	0	4	3	227	9	0	239	894
07:30 AM	6	0	5	0	11	12	579	0	7	598	3	1	2	0	6	4	290	12	1	307	922
07:45 AM	5	0	6	0	11	14	554	3	12	583	2	0	4	0	6	4	310	11	2	327	927
Total Volume	20	0	24	0	44	52	2394	6	26	2478	9	9	8	0	26	17	1026	35	3	1081	3629
% App. Total																					
PHF	.833	.000	.857	.000	1.00	.929	.935	.500	.542	.943	.750	.321	.500	.000	.650	.708	.827	.729	.375	.826	.979
Cars	14	0	17	0	31	48	2350	4	26	2428	2	4	2	0	8	8	962	32	3	1005	3472
% Cars	70.0	0	70.8	0	70.5	92.3	98.2	66.7	100	98.0	22.2	44.4	25.0	0	30.8	47.1	93.8	91.4	100	93.0	95.7
Heavy Vehicles	30.0	0	29.2	0	29.5	7.7	1.8	33.3	0	2.0	77.8	55.6	75.0	0	69.2	52.9	6.2	8.6	0	7.0	4.3



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	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
07:00 AM	4	0	4	0	14	625	0	2	0	3	0	0	4	188	2	0	846
07:15 AM	2	0	5	0	11	605	1	5	0	0	0	0	1	219	9	0	858
07:30 AM	3	0	3	0	11	571	0	7	1	1	1	0	2	265	12	1	878
07:45 AM	5	0	5	0	12	549	3	12	1	0	1	0	1	290	9	2	890
Total	14	0	17	0	48	2350	4	26	2	4	2	0	8	962	32	3	3472
08:00 AM	3	0	12	0	11	537	3	15	3	0	1	0	3	228	8	0	824
08:15 AM	2	0	6	0	12	532	4	6	3	3	2	0	2	273	10	0	855
08:30 AM	4	0	9	0	12	429	2	9	2	0	2	0	3	253	12	0	737
08:45 AM	2	0	6	0	11	444	0	10	4	0	0	0	0	277	9	0	763
Total	11	0	33	0	46	1942	9	40	12	3	5	0	8	1031	39	0	3179
Grand Total	25	0	50	0	94	4292	13	66	14	7	7	0	16	1993	71	3	6651
Apprch %	33.3	0	66.7	0	2.1	96.1	0.3	1.5	50	25	25	0	0.8	95.7	3.4	0.1	
Total %	0.4	0	0.8	0	1.4	64.5	0.2	1	0.2	0.1	0.1	0	0.2	30	1.1	0	

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	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total		
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																						
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07:00 AM	4	0	4	0	8	14	625	0	2	641	0	3	0	0	3	4	188	2	0	194	846	
07:15 AM	2	0	5	0	7	11	605	1	5	622	0	0	0	0	0	1	219	9	0	229	858	
07:30 AM	3	0	3	0	6	11	571	0	7	589	1	1	1	0	3	2	265	12	1	280	878	
07:45 AM	5	0	5	0	10	12	549	3	12	576	1	0	1	0	2	1	290	9	2	302	890	
Total Volume	14	0	17	0	31	48	2350	4	26	2428	2	4	2	0	8	8	962	32	3	1005	3472	
PHF	.700	.000	.850	.000	.775	.857	.940	.333	.542	.947	.500	.333	.500	.000	.667	.500	.829	.667	.375	.832	.975	



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Start Time																	
07:00 AM	0	0	3	0	0	15	1	0	2	4	1	0	2	11	1	0	40
07:15 AM	3	0	1	0	1	16	1	0	2	1	1	0	2	8	0	0	36
07:30 AM	3	0	2	0	1	8	0	0	2	0	1	0	2	25	0	0	44
07:45 AM	0	0	1	0	2	5	0	0	1	0	3	0	3	20	2	0	37
Total	6	0	7	0	4	44	2	0	7	5	6	0	9	64	3	0	157
08:00 AM	0	0	0	0	2	8	2	0	1	0	0	0	3	8	2	0	26
08:15 AM	0	0	2	0	0	17	0	1	0	0	0	0	1	14	1	0	36
08:30 AM	0	0	0	0	1	15	1	1	1	0	0	0	1	13	1	0	34
08:45 AM	0	0	0	0	0	13	2	0	0	0	0	0	2	10	0	0	27
Total	0	0	2	0	3	53	5	2	2	0	0	0	7	45	4	0	123
Grand Total	6	0	9	0	7	97	7	2	9	5	6	0	16	109	7	0	280
Apprch %	40	0	60	0	6.2	85.8	6.2	1.8	45	25	30	0	12.1	82.6	5.3	0	
Total %	2.1	0	3.2	0	2.5	34.6	2.5	0.7	3.2	1.8	2.1	0	5.7	38.9	2.5	0	

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07:15 AM	3	0	1	0	4	1	16	1	0	18	2	1	1	0	4	2	8	0	0	10	36
07:30 AM	3	0	2	0	5	1	8	0	0	9	2	0	1	0	3	2	25	0	0	27	44
07:45 AM	0	0	1	0	1	2	5	0	0	7	1	0	3	0	4	3	20	2	0	25	37
Total Volume	6	0	7	0	13	4	44	2	0	50	7	5	6	0	18	9	64	3	0	76	157
% App. Total	46.2	0	53.8	0		8	88	4	0		38.9	27.8	33.3	0		11.8	84.2	3.9	0		
PHF	.500	.000	.583	.000	.650	.500	.688	.500	.000	.694	.875	.313	.500	.000	.643	.750	.640	.375	.000	.704	.892



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D A T A
INDUSTRIES,LLC

P.O. Box 301 Berlin, MA 01503
Office: 508.481.3999 Fax: 508.545.1234
Email: datarequests@pdillc.com

N/S: Shepard Street/ Marine Boulevard
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 A
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Peds and Bikes

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	Peds EB	Peds WB	Right	Thru	Left	Peds SB	Peds NB	Right	Thru	Left	Peds WB	Peds EB	Right	Thru	Left	Peds NB	Peds SB	Int. Total
07:00 AM	0	0	0	2	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
07:30 AM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	3
07:45 AM	0	0	0	2	1	0	0	0	0	0	0	0	0	0	2	0	0	0	0	1	6
Total	0	0	0	4	4	0	0	0	0	0	0	0	0	0	3	0	0	0	1	1	13
08:00 AM	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
Total	0	0	0	2	2	0	0	0	0	0	0	0	0	0	1	0	0	0	2	0	7
Grand Total	0	0	0	6	6	0	0	0	0	0	0	0	0	0	4	0	0	0	3	1	20
Apprch %	0	0	0	50	50	0	0	0	0	0	0	0	0	0	100	0	0	0	75	25	
Total %	0	0	0	30	30	0	0	0	0	0	0	0	0	0	20	0	0	0	15	5	

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West								
	Right	Thru	Left	Peds EB	Peds WB	App. Total	Right	Thru	Left	Peds SB	Peds NB	App. Total	Right	Thru	Left	Peds WB	Peds EB	App. Total	Right	Thru	Left	Peds SB	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																								
Peak Hour for Entire Intersection Begins at 07:30 AM																								
07:30 AM	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
07:45 AM	0	0	0	2	1	3	0	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	1	1
08:00 AM	0	0	0	2	2	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
Total Volume	0	0	0	4	5	9	0	0	0	0	0	0	0	0	0	0	0	2	2	0	0	3	1	15
% App. Total	0	0	0	44.4	55.6		0	0	0	0	0	0	0	0	0	0	0	100	0	0	0	75	25	
PHF	.000	.000	.000	.500	.625	.563	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250	.250	.000	.000	.375	.250	.500



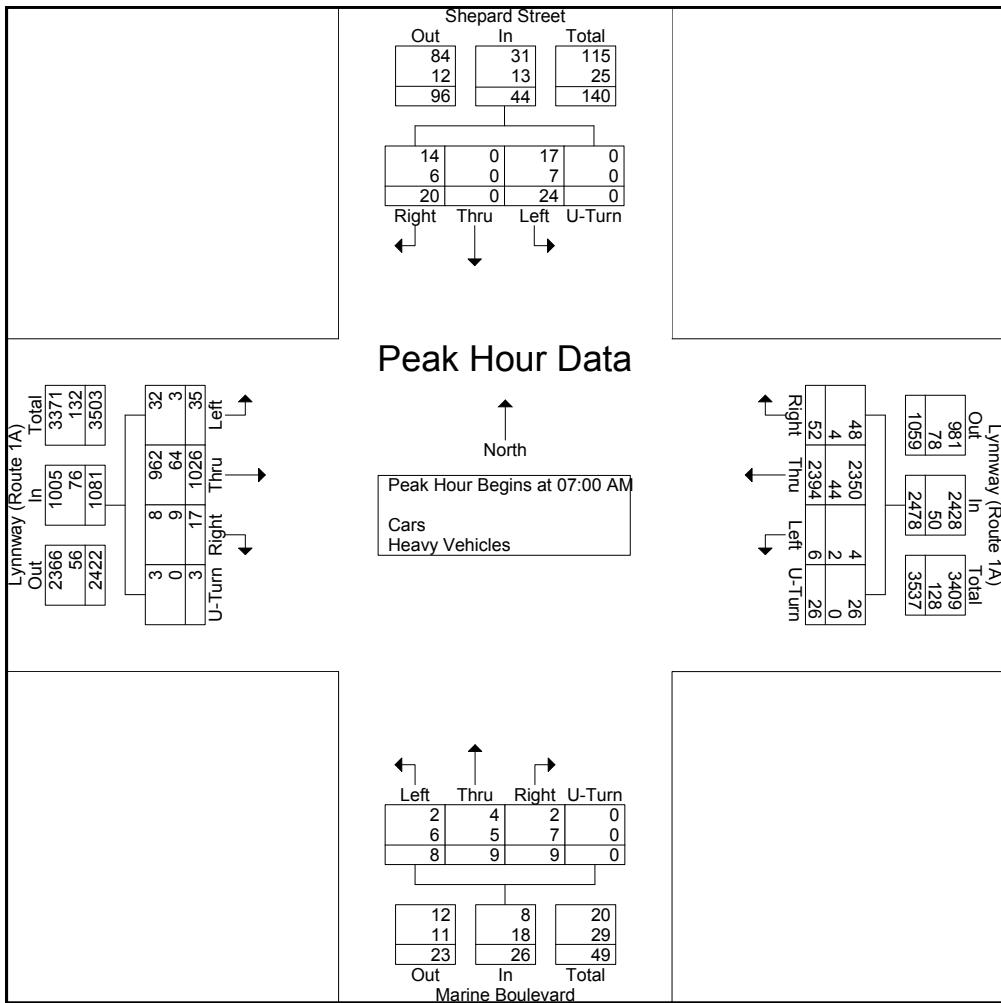
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Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																					
07:00 AM	4	0	7	0	11	14	640	1	2	657	2	7	1	0	10	6	199	3	0	208	886
07:15 AM	5	0	6	0	11	12	621	2	5	640	2	1	1	0	4	3	227	9	0	239	894
07:30 AM	6	0	5	0	11	12	579	0	7	598	3	1	2	0	6	4	290	12	1	307	922
07:45 AM	5	0	6	0	11	14	554	3	12	583	2	0	4	0	6	4	310	11	2	327	927
Total Volume % App. Total	20	0	24	0	44	52	2394	6	26	2478	9	9	8	0	26	17	1026	35	3	1081	3629
PHF	.833	.000	.857	.000	1.00	.929	.935	.500	.542	.943	.750	.321	.500	.000	.650	.708	.827	.729	.375	.826	.979
Cars	14	0	17	0	31	48	2350	4	26	2428	2	4	2	0	8	8	962	32	3	1005	3472
% Cars	70.0	0	70.8	0	70.5	92.3	98.2	66.7	100	98.0	22.2	44.4	25.0	0	30.8	47.1	93.8	91.4	100	93.0	95.7
Heavy Vehicles % Heavy Vehicles	30.0	0	29.2	0	29.5	7.7	1.8	33.3	0	2.0	77.8	55.6	75.0	0	69.2	52.9	6.2	8.6	0	7.0	4.3





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File Name : 144179 AA
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Groups Printed- Cars - Heavy Vehicles

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
04:00 PM	5	0	6	0	10	293	0	8	3	0	2	1	7	436	16	2	789
04:15 PM	6	0	12	0	15	314	3	8	2	0	13	1	3	478	16	4	875
04:30 PM	7	0	15	0	12	286	1	6	2	1	10	0	4	531	25	2	902
04:45 PM	2	0	14	0	17	281	1	16	1	1	2	0	5	585	18	2	945
Total	20	0	47	0	54	1174	5	38	8	2	27	2	19	2030	75	10	3511
05:00 PM	6	0	11	0	7	324	0	12	4	6	8	0	8	575	30	2	993
05:15 PM	0	0	8	0	10	311	0	5	3	0	0	0	0	582	31	4	954
05:30 PM	6	0	12	0	16	261	0	8	0	0	2	0	2	561	21	3	892
05:45 PM	6	0	6	0	9	245	0	10	1	1	1	0	1	603	21	2	906
Total	18	0	37	0	42	1141	0	35	8	7	11	0	11	2321	103	11	3745
Grand Total	38	0	84	0	96	2315	5	73	16	9	38	2	30	4351	178	21	7256
Apprch %	31.1	0	68.9	0	3.9	93	0.2	2.9	24.6	13.8	58.5	3.1	0.7	95	3.9	0.5	
Total %	0.5	0	1.2	0	1.3	31.9	0.1	1	0.2	0.1	0.5	0	0.4	60	2.5	0.3	
Cars	37	0	82	0	95	2256	0	72	16	9	36	2	21	4271	173	21	7091
% Cars	97.4	0	97.6	0	99	97.5	0	98.6	100	100	94.7	100	70	98.2	97.2	100	97.7
Heavy Vehicles	1	0	2	0	1	59	5	1	0	0	2	0	9	80	5	0	165
% Heavy Vehicles	2.6	0	2.4	0	1	2.5	100	1.4	0	0	5.3	0	30	1.8	2.8	0	2.3

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total				
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn		
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:30 PM																					
04:30 PM	7	0	15	0	22	12	286	1	6	305	2	1	10	0	13	4	531	25	2	562	902
04:45 PM	2	0	14	0	16	17	281	1	16	315	1	1	2	0	4	5	585	18	2	610	945
05:00 PM	6	0	11	0	17	7	324	0	12	343	4	6	8	0	18	8	575	30	2	615	993
05:15 PM	0	0	8	0	8	10	311	0	5	326	3	0	0	0	3	0	582	31	4	617	954
Total Volume	15	0	48	0	63	46	1202	2	39	1289	10	8	20	0	38	17	2273	104	10	2404	3794
% App. Total	.536	.000	.800	.000	.716	.676	.927	.500	.609	.940	.625	.333	.500	.000	.528	.531	.971	.839	.625	.974	.955
Cars	15	0	48	0	63	45	1170	0	39	1254	10	8	19	0	37	13	2232	100	10	2355	3709
% Cars	100	0	100	0	100	97.8	97.3	0	100	97.3	100	100	95.0	0	97.4	76.5	98.2	96.2	100	98.0	97.8
Heavy Vehicles	0	0	0	0	0	2.2	2.7	100	0	2.7	0	0	5.0	0	2.6	23.5	1.8	3.8	0	2.0	2.2



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Page No : 1

Groups Printed- Cars

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
04:00 PM	4	0	6	0	10	285	0	8	3	0	2	1	4	430	16	2	771
04:15 PM	6	0	11	0	15	304	0	8	2	0	12	1	2	464	16	4	845
04:30 PM	7	0	15	0	12	275	0	6	2	1	10	0	2	520	23	2	875
04:45 PM	2	0	14	0	17	277	0	16	1	1	1	0	3	573	17	2	924
Total	19	0	46	0	54	1141	0	38	8	2	25	2	11	1987	72	10	3415
05:00 PM	6	0	11	0	6	315	0	12	4	6	8	0	8	563	29	2	970
05:15 PM	0	0	8	0	10	303	0	5	3	0	0	0	0	576	31	4	940
05:30 PM	6	0	11	0	16	253	0	7	0	0	2	0	1	550	21	3	870
05:45 PM	6	0	6	0	9	244	0	10	1	1	1	0	1	595	20	2	896
Total	18	0	36	0	41	1115	0	34	8	7	11	0	10	2284	101	11	3676
Grand Total	37	0	82	0	95	2256	0	72	16	9	36	2	21	4271	173	21	7091
Apprch %	31.1	0	68.9	0	3.9	93.1	0	3	25.4	14.3	57.1	3.2	0.5	95.2	3.9	0.5	
Total %	0.5	0	1.2	0	1.3	31.8	0	1	0.2	0.1	0.5	0	0.3	60.2	2.4	0.3	

	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					Int. Total	
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total		
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																						
Peak Hour for Entire Intersection Begins at 04:30 PM																						
04:30 PM	7	0	15	0	22	12	275	0	6	293	2	1	10	0	13	2	520	23	2	547	875	
04:45 PM	2	0	14	0	16	17	277	0	16	310	1	1	1	0	3	3	573	17	2	595	924	
05:00 PM	6	0	11	0	17	6	315	0	12	333	4	6	8	0	18	8	563	29	2	602	970	
05:15 PM	0	0	8	0	8	10	303	0	5	318	3	0	0	0	3	0	576	31	4	611	940	
Total Volume	15	0	48	0	63	45	1170	0	39	1254	10	8	19	0	37	13	2232	100	10	2355	3709	
% App. Total	PHF	.536	.000	.800	.000	.716	.662	.929	.000	.609	.941	.625	.333	.475	.000	.514	.406	.969	.806	.625	.964	.956



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Groups Printed- Heavy Vehicles

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
04:00 PM	1	0	0	0	0	8	0	0	0	0	0	0	3	6	0	0	18
04:15 PM	0	0	1	0	0	10	3	0	0	0	1	0	1	14	0	0	30
04:30 PM	0	0	0	0	0	11	1	0	0	0	0	0	2	11	2	0	27
04:45 PM	0	0	0	0	0	4	1	0	0	0	1	0	2	12	1	0	21
Total	1	0	1	0	0	33	5	0	0	0	2	0	8	43	3	0	96
05:00 PM	0	0	0	0	1	9	0	0	0	0	0	0	0	12	1	0	23
05:15 PM	0	0	0	0	0	8	0	0	0	0	0	0	0	6	0	0	14
05:30 PM	0	0	1	0	0	8	0	1	0	0	0	0	1	11	0	0	22
05:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	8	1	0	10
Total	0	0	1	0	1	26	0	1	0	0	0	0	1	37	2	0	69
Grand Total	1	0	2	0	1	59	5	1	0	0	2	0	9	80	5	0	165
Apprch %	33.3	0	66.7	0	1.5	89.4	7.6	1.5	0	0	100	0	9.6	85.1	5.3	0	
Total %	0.6	0	1.2	0	0.6	35.8	3	0.6	0	0	1.2	0	5.5	48.5	3	0	

	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					Int. Total
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	
Start Time																					
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:15 PM																					
04:15 PM	0	0	1	0	1	0	10	3	0	13	0	0	1	0	1	1	14	0	0	15	30
04:30 PM	0	0	0	0	0	0	11	1	0	12	0	0	0	0	0	2	11	2	0	15	27
04:45 PM	0	0	0	0	0	0	4	1	0	5	0	0	1	0	1	2	12	1	0	15	21
05:00 PM	0	0	0	0	0	1	9	0	0	10	0	0	0	0	0	0	12	1	0	13	23
Total Volume	0	0	1	0	1	1	34	5	0	40	0	0	2	0	2	5	49	4	0	58	101
% App. Total	0	0	100	0		2.5	85	12.5	0		0	0	100	0		8.6	84.5	6.9	0		
PHF	.000	.000	.250	.000	.250	.250	.773	.417	.000	.769	.000	.000	.500	.000	.500	.625	.875	.500	.000	.967	.842



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	Right	Thru	Left	Peds EB	Peds WB	Right	Thru	Left	Peds SB	Peds NB	Right	Thru	Left	Peds WB	Peds EB	Right	Thru	Left	Peds NB	Peds SB	Int. Total
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2
04:30 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	4	0	6
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	3	0	4
Total	0	0	0	0	0	0	1	0	0	0	0	0	0	0	2	0	0	0	7	3	13
05:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	2
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	1	1	4
Total	0	0	0	1	1	0	0	0	0	0	0	0	0	2	1	0	0	0	1	1	7
Grand Total	0	0	0	1	1	0	1	0	0	0	0	0	0	2	3	0	0	0	8	4	20
Apprch %	0	0	0	50	50	0	100	0	0	0	0	0	0	40	60	0	0	0	66.7	33.3	
Total %	0	0	0	5	5	0	5	0	0	0	0	0	0	10	15	0	0	0	40	20	

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West									
	Right	Thru	Left	Peds EB	Peds WB	App. Total	Right	Thru	Left	Peds SB	Peds NB	App. Total	Right	Thru	Left	Peds WB	Peds EB	App. Total	Right	Thru	Left	Peds NB	Peds SB	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																									
Peak Hour for Entire Intersection Begins at 04:15 PM																									
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2
04:30 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	1	1	0	0	0	4	0	4	6
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	3	0	3
05:00 PM	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	2
Total Volume	0	0	0	0	1	1	0	1	0	0	0	1	0	0	0	1	2	3	0	0	0	7	2	9	14
% App. Total	0	0	0	0	100	0	100	0	0	0	0	0	0	0	33.3	66.7	0	0	0	77.8	22.2				
PHF	.000	.000	.000	.000	.250	.250	.000	.250	.000	.000	.000	.250	.000	.000	.250	.500	.750	.000	.000	.000	.438	.250	.563	.583	



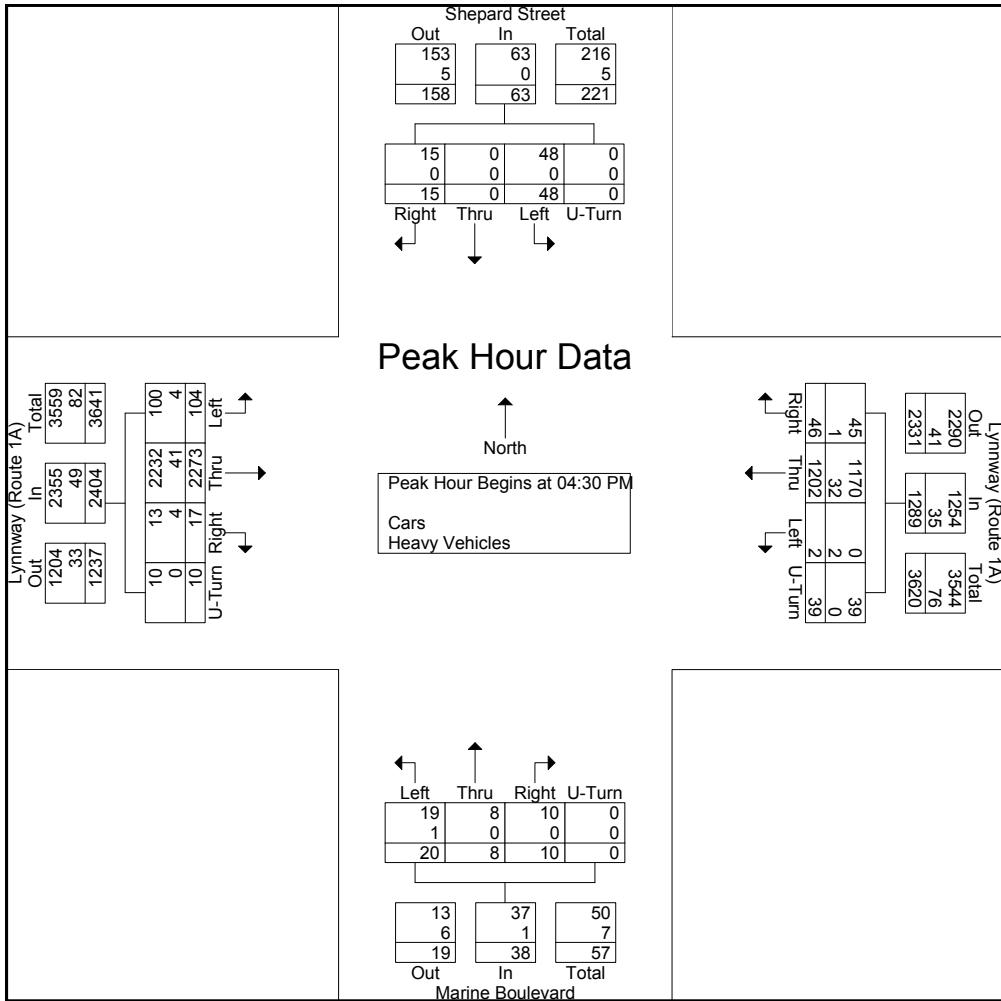
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INDUSTRIES, LLC

P.O. Box 301 Berlin, MA 01503
Office: 508.481.3999 Fax: 508.545.1234
Email: datarequests@pdillc.com

N/S: Shepard Street/ Marine Boulevard
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 AA
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
04:30 PM	7	0	15	0	22	12	286	1	6	305	2	1	10	0	13	4	531	25	2	562	902
04:45 PM	2	0	14	0	16	17	281	1	16	315	1	1	2	0	4	5	585	18	2	610	945
05:00 PM	6	0	11	0	17	7	324	0	12	343	4	6	8	0	18	8	575	30	2	615	993
05:15 PM	0	0	8	0	8	10	311	0	5	326	3	0	0	0	3	0	582	31	4	617	954
Total Volume % App. Total	15	0	48	0	63	46	1202	2	39	1289	10	8	20	0	38	17	2273	104	10	2404	3794
PHF	.536	.000	.800	.000	.716	.676	.927	.500	.609	.940	.625	.333	.500	.000	.528	.531	.971	.839	.625	.974	.955
Cars	15	0	48	0	63	45	1170	0	39	1254	10	8	19	0	37	13	2232	100	10	2355	3709
% Cars	100	0	100	0	100	97.8	97.3	0	100	97.3	100	100	95.0	0	97.4	76.5	98.2	96.2	100	98.0	97.8
Heavy Vehicles % Heavy Vehicles	0	0	0	0	0	2.2	2.7	100	0	2.7	0	0	5.0	0	2.6	23.5	1.8	3.8	0	2.0	2.2





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File Name : 144179 B
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Groups Printed- Cars - Heavy Vehicles

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
07:00 AM	6	0	0	0	19	675	0	0	14	0	0	0	18	200	0	0	932
07:15 AM	6	0	0	0	38	640	0	0	14	0	0	0	20	221	0	0	939
07:30 AM	15	0	0	0	46	591	0	0	23	0	0	0	25	285	0	0	985
07:45 AM	14	0	0	0	44	596	0	0	18	0	0	0	29	300	0	0	1001
Total	41	0	0	0	147	2502	0	0	69	0	0	0	92	1006	0	0	3857
08:00 AM	9	0	0	0	40	567	0	0	19	0	0	0	20	251	0	0	906
08:15 AM	14	0	0	0	30	567	0	0	20	0	0	0	34	268	0	0	933
08:30 AM	10	0	0	0	34	483	0	0	17	0	0	0	18	276	0	0	838
08:45 AM	11	0	0	0	36	468	0	0	16	0	0	0	27	277	0	0	835
Total	44	0	0	0	140	2085	0	0	72	0	0	0	99	1072	0	0	3512
Grand Total	85	0	0	0	287	4587	0	0	141	0	0	0	191	2078	0	0	7369
Apprch %	100	0	0	0	5.9	94.1	0	0	100	0	0	0	8.4	91.6	0	0	
Total %	1.2	0	0	0	3.9	62.2	0	0	1.9	0	0	0	2.6	28.2	0	0	
Cars	78	0	0	0	268	4480	0	0	129	0	0	0	177	1953	0	0	7085
% Cars	91.8	0	0	0	93.4	97.7	0	0	91.5	0	0	0	92.7	94	0	0	96.1
Heavy Vehicles	7	0	0	0	19	107	0	0	12	0	0	0	14	125	0	0	284
% Heavy Vehicles	8.2	0	0	0	6.6	2.3	0	0	8.5	0	0	0	7.3	6	0	0	3.9

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total	
Start Time																	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:00 AM																	
07:00 AM	6	0	0	0	6	19	675	0	0	694	14	0	0	0	14	218	932
07:15 AM	6	0	0	0	6	38	640	0	0	678	14	0	0	0	14	221	939
07:30 AM	15	0	0	0	15	46	591	0	0	637	23	0	0	0	23	285	310
07:45 AM	14	0	0	0	14	44	596	0	0	640	18	0	0	0	18	300	329
Total Volume	41	0	0	0	41	147	2502	0	0	2649	69	0	0	0	69	1006	1098
% App. Total	.683	.000	.000	.000	.683	.799	.927	.000	.000	.954	.750	.000	.000	.000	.750	.838	.963
PHF	.683	.000	.000	.000	.683	.799	.927	.000	.000	.954	.750	.000	.000	.000	.750	.838	.963
Cars	38	0	0	0	38	138	2453	0	0	2591	63	0	0	0	63	930	1017
% Cars	92.7	0	0	0	92.7	93.9	98.0	0	0	97.8	91.3	0	0	0	91.3	92.4	96.2
Heavy Vehicles	7.3	0	0	0	7.3	6.1	2.0	0	0	2.2	8.7	0	0	0	8.7	7.6	3.8
% Heavy Vehicles	7.3	0	0	0	7.3	6.1	2.0	0	0	2.2	8.7	5.4	7.6	0	0	7.4	3.8



PRECISION
D A T A
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N/S: Blossom Street/ Blossom Street Ext
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 B
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Cars

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
07:00 AM	5	0	0	0	15	659	0	0	12	0	0	0	18	183	0	0	892
07:15 AM	5	0	0	0	38	624	0	0	14	0	0	0	18	209	0	0	908
07:30 AM	15	0	0	0	42	581	0	0	20	0	0	0	23	257	0	0	938
07:45 AM	13	0	0	0	43	589	0	0	17	0	0	0	28	281	0	0	971
Total	38	0	0	0	138	2453	0	0	63	0	0	0	87	930	0	0	3709
08:00 AM	9	0	0	0	37	556	0	0	18	0	0	0	20	241	0	0	881
08:15 AM	12	0	0	0	28	550	0	0	20	0	0	0	30	255	0	0	895
08:30 AM	9	0	0	0	32	466	0	0	14	0	0	0	14	260	0	0	795
08:45 AM	10	0	0	0	33	455	0	0	14	0	0	0	26	267	0	0	805
Total	40	0	0	0	130	2027	0	0	66	0	0	0	90	1023	0	0	3376
Grand Total	78	0	0	0	268	4480	0	0	129	0	0	0	177	1953	0	0	7085
Apprch %	100	0	0	0	5.6	94.4	0	0	100	0	0	0	8.3	91.7	0	0	
Total %	1.1	0	0	0	3.8	63.2	0	0	1.8	0	0	0	2.5	27.6	0	0	

	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					Int. Total	
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total		
Start Time																						
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																						
Peak Hour for Entire Intersection Begins at 07:00 AM																						
07:00 AM	5	0	0	0	5	15	659	0	0	674	12	0	0	0	12	18	183	0	0	201	892	
07:15 AM	5	0	0	0	5	38	624	0	0	662	14	0	0	0	14	18	209	0	0	227	908	
07:30 AM	15	0	0	0	15	42	581	0	0	623	20	0	0	0	20	23	257	0	0	280	938	
07:45 AM	13	0	0	0	13	43	589	0	0	632	17	0	0	0	17	28	281	0	0	309	971	
Total Volume	38	0	0	0	38	138	2453	0	0	2591	63	0	0	0	63	87	930	0	0	1017	3709	
% App. Total	PHF	.633	.000	.000	.000	.633	.802	.931	.000	.000	.961	.788	.000	.000	.000	.788	.777	.827	.000	.000	.823	.955



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City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 B
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Heavy Vehicles

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
07:00 AM	1	0	0	0	4	16	0	0	2	0	0	0	0	17	0	0	40
07:15 AM	1	0	0	0	0	16	0	0	0	0	0	0	2	12	0	0	31
07:30 AM	0	0	0	0	4	10	0	0	3	0	0	0	2	28	0	0	47
07:45 AM	1	0	0	0	1	7	0	0	1	0	0	0	1	19	0	0	30
Total	3	0	0	0	9	49	0	0	6	0	0	0	5	76	0	0	148
08:00 AM	0	0	0	0	3	11	0	0	1	0	0	0	0	10	0	0	25
08:15 AM	2	0	0	0	2	17	0	0	0	0	0	0	4	13	0	0	38
08:30 AM	1	0	0	0	2	17	0	0	3	0	0	0	4	16	0	0	43
08:45 AM	1	0	0	0	3	13	0	0	2	0	0	0	1	10	0	0	30
Total	4	0	0	0	10	58	0	0	6	0	0	0	9	49	0	0	136
Grand Total	7	0	0	0	19	107	0	0	12	0	0	0	14	125	0	0	284
Apprch %	100	0	0	0	15.1	84.9	0	0	100	0	0	0	10.1	89.9	0	0	
Total %	2.5	0	0	0	6.7	37.7	0	0	4.2	0	0	0	4.9	44	0	0	

	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					Int. Total
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	
Start Time																					
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:00 AM																					
07:00 AM	1	0	0	0	1	4	16	0	0	20	2	0	0	0	2	0	17	0	0	17	40
07:15 AM	1	0	0	0	1	0	16	0	0	16	0	0	0	0	0	2	12	0	0	14	31
07:30 AM	0	0	0	0	0	4	10	0	0	14	3	0	0	0	3	2	28	0	0	30	47
07:45 AM	1	0	0	0	1	1	7	0	0	8	1	0	0	0	1	1	19	0	0	20	30
Total Volume	3	0	0	0	3	9	49	0	0	58	6	0	0	0	6	5	76	0	0	81	148
% App. Total	100	0	0	0		15.5	84.5	0	0		100	0	0	0		6.2	93.8	0	0		
PHF	.750	.000	.000	.000	.750	.563	.766	.000	.000	.725	.500	.000	.000	.000	.500	.625	.679	.000	.000	.675	.787



**PRECISION
D A T A
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N/S: Blossom Street/ Blossom Street Ext
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Client: VHB/K. Keen

File Name : 144179 B
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Peds and Bikes

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	Peds EB	Peds WB	Right	Thru	Left	Peds SB	Peds NB	Right	Thru	Left	Peds WB	Peds EB	Right	Thru	Left	Peds NB	Peds SB	Int. Total
07:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:15 AM	0	0	0	2	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	3
07:30 AM	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
07:45 AM	0	0	0	3	1	0	0	0	0	1	0	0	0	0	0	0	0	0	2	1	8
Total	0	0	0	5	5	0	0	0	0	1	0	0	0	1	0	0	0	0	2	1	15
08:00 AM	0	0	0	2	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	7
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
08:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Total	0	0	0	3	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	9
Grand Total	0	0	0	8	10	0	0	0	0	1	0	0	0	1	0	0	0	0	2	2	24
Apprch %	0	0	0	44.4	55.6	0	0	0	0	100	0	0	0	100	0	0	0	0	50	50	
Total %	0	0	0	33.3	41.7	0	0	0	0	4.2	0	0	0	4.2	0	0	0	0	8.3	8.3	

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West									
	Right	Thru	Left	Peds EB	Peds WB	App. Total	Right	Thru	Left	Peds SB	Peds NB	App. Total	Right	Thru	Left	Peds WB	Peds EB	App. Total	Right	Thru	Left	Peds NB	Peds SB	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																									
Peak Hour for Entire Intersection Begins at 07:15 AM																									
07:15 AM	0	0	0	2	0	2	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	3	
07:30 AM	0	0	0	0	3	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
07:45 AM	0	0	0	3	1	4	0	0	0	0	1	1	0	0	0	0	0	0	0	0	2	1	8		
08:00 AM	0	0	0	2	4	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	7		
Total Volume	0	0	0	7	8	15	0	0	0	0	1	1	0	0	0	1	0	1	0	0	0	2	4	21	
% App. Total	0	0	0	46.7	53.3		0	0	0	0	100	0	0	0	100	0	0	0	0	50	50				
PHF	.000	.000	.000	.583	.500	.625	.000	.000	.000	.000	.250	.250	.000	.000	.000	.250	.000	.250	.000	.000	.000	.250	.500	.333	.656



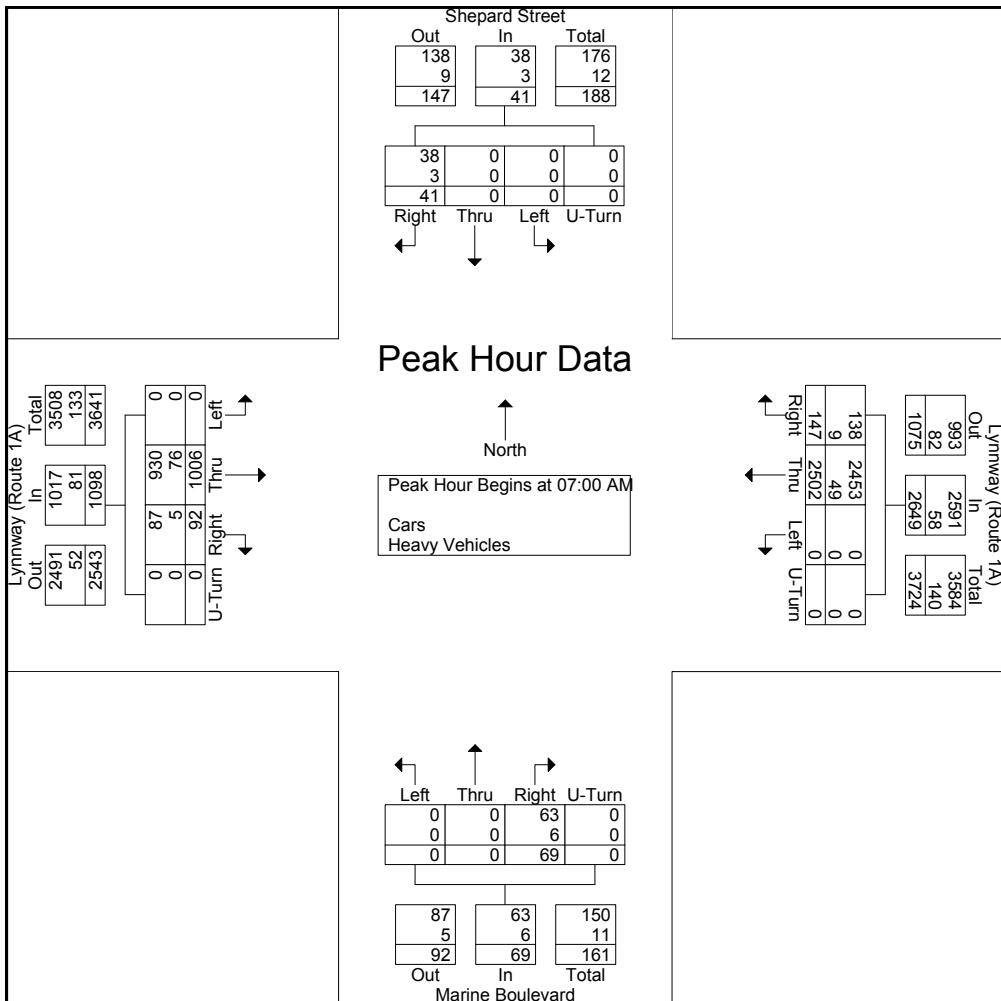
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	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																					
07:00 AM	6	0	0	0	6	19	675	0	0	694	14	0	0	0	14	18	200	0	0	218	932
07:15 AM	6	0	0	0	6	38	640	0	0	678	14	0	0	0	14	20	221	0	0	241	939
07:30 AM	15	0	0	0	15	46	591	0	0	637	23	0	0	0	23	25	285	0	0	310	985
07:45 AM	14	0	0	0	14	44	596	0	0	640	18	0	0	0	18	29	300	0	0	329	1001
Total Volume % App. Total	41	0	0	0	41	147	2502	0	0	2649	69	0	0	0	69	92	1006	0	0	1098	3857
PHF	.683	.000	.000	.000	.683	.799	.927	.000	.000	.954	.750	.000	.000	.000	.750	.793	.838	.000	.000	.834	.963
Cars	38	0	0	0	38	138	2453	0	0	2591	63	0	0	0	63	87	930	0	0	1017	3709
% Cars	92.7	0	0	0	92.7	93.9	98.0	0	0	97.8	91.3	0	0	0	91.3	94.6	92.4	0	0	92.6	96.2
Heavy Vehicles	7.3	0	0	0	7.3	6.1	2.0	0	0	2.2	8.7	0	0	0	8.7	5.4	7.6	0	0	7.4	3.8





PRECISION
DATA
INDUSTRIES, LLC

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Groups Printed- Cars - Heavy Vehicles

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
Start Time	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
04:00 PM	10	0	0	0	32	328	0	0	13	0	0	0	12	450	0	0	845
04:15 PM	15	0	0	0	28	331	0	0	10	0	0	0	10	484	0	0	878
04:30 PM	17	0	0	0	29	309	0	0	15	0	0	0	13	550	0	0	933
04:45 PM	7	0	0	0	35	325	0	0	15	0	0	0	12	579	0	0	973
Total	49	0	0	0	124	1293	0	0	53	0	0	0	47	2063	0	0	3629
05:00 PM	14	0	0	0	52	360	0	0	14	0	0	0	9	595	0	0	1044
05:15 PM	5	0	0	0	40	340	0	0	12	0	0	0	11	594	0	0	1002
05:30 PM	16	0	0	0	38	314	0	0	9	0	0	0	6	591	0	0	974
05:45 PM	11	0	0	0	23	264	0	0	9	0	0	0	8	624	0	0	939
Total	46	0	0	0	153	1278	0	0	44	0	0	0	34	2404	0	0	3959
Grand Total	95	0	0	0	277	2571	0	0	97	0	0	0	81	4467	0	0	7588
Apprch %	100	0	0	0	9.7	90.3	0	0	100	0	0	0	1.8	98.2	0	0	
Total %	1.3	0	0	0	3.7	33.9	0	0	1.3	0	0	0	1.1	58.9	0	0	
Cars	93	0	0	0	264	2495	0	0	95	0	0	0	81	4405	0	0	7433
% Cars	97.9	0	0	0	95.3	97	0	0	97.9	0	0	0	100	98.6	0	0	98
Heavy Vehicles	2	0	0	0	13	76	0	0	2	0	0	0	0	62	0	0	155
% Heavy Vehicles	2.1	0	0	0	4.7	3	0	0	2.1	0	0	0	0	1.4	0	0	2

	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West						
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																						
Peak Hour for Entire Intersection Begins at 04:45 PM																						
04:45 PM	7	0	0	0	7	35	325	0	0	360	15	0	0	0	15	12	579	0	0	591	973	
05:00 PM	14	0	0	0	14	52	360	0	0	412	14	0	0	0	14	9	595	0	0	604	1044	
05:15 PM	5	0	0	0	5	40	340	0	0	380	12	0	0	0	12	11	594	0	0	605	1002	
05:30 PM	16	0	0	0	16	38	314	0	0	352	9	0	0	0	9	6	591	0	0	597	974	
Total Volume	42	0	0	0	42	165	1339	0	0	1504	50	0	0	0	50	38	2359	0	0	2397	3993	
% App. Total	.656	.000	.000	.000	.656	.793	.930	.000	.000	.913	.833	.000	.000	.000	.833	.792	.991	.000	.000	.990	.956	
Cars	40	0	0	0	40	159	1308	0	0	1467	49	0	0	0	49	38	2329	0	0	2367	3923	
% Cars	95.2	0	0	0	95.2	96.4	97.7	0	0	97.5	98.0	0	0	0	98.0	100	98.7	0	0	98.7	98.2	
Heavy Vehicles	4.8	0	0	0	4.8	3.6	2.3	0	0	2.5	2.0	0	0	0	2.0	0	1.3	0	0	1.3	1.8	



PRECISION
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INDUSTRIES,LLC

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N/S: Blossom Street/ Blossom Street Ext
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 BB
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Cars

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
04:00 PM	10	0	0	0	30	310	0	0	13	0	0	0	12	447	0	0	822
04:15 PM	15	0	0	0	26	319	0	0	9	0	0	0	10	472	0	0	851
04:30 PM	17	0	0	0	26	296	0	0	15	0	0	0	13	542	0	0	909
04:45 PM	6	0	0	0	34	320	0	0	15	0	0	0	12	574	0	0	961
Total	48	0	0	0	116	1245	0	0	52	0	0	0	47	2035	0	0	3543
05:00 PM	13	0	0	0	49	352	0	0	14	0	0	0	9	586	0	0	1023
05:15 PM	5	0	0	0	38	331	0	0	11	0	0	0	11	590	0	0	986
05:30 PM	16	0	0	0	38	305	0	0	9	0	0	0	6	579	0	0	953
05:45 PM	11	0	0	0	23	262	0	0	9	0	0	0	8	615	0	0	928
Total	45	0	0	0	148	1250	0	0	43	0	0	0	34	2370	0	0	3890
Grand Total	93	0	0	0	264	2495	0	0	95	0	0	0	81	4405	0	0	7433
Apprch %	100	0	0	0	9.6	90.4	0	0	100	0	0	0	1.8	98.2	0	0	
Total %	1.3	0	0	0	3.6	33.6	0	0	1.3	0	0	0	1.1	59.3	0	0	

	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					Int. Total	
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total		
Start Time																						
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																						
Peak Hour for Entire Intersection Begins at 04:45 PM																						
04:45 PM	6	0	0	0	6	34	320	0	0	354	15	0	0	0	15	12	574	0	0	586	961	
05:00 PM	13	0	0	0	13	49	352	0	0	401	14	0	0	0	14	9	586	0	0	595	1023	
05:15 PM	5	0	0	0	5	38	331	0	0	369	11	0	0	0	11	11	590	0	0	601	986	
05:30 PM	16	0	0	0	16	38	305	0	0	343	9	0	0	0	9	6	579	0	0	585	953	
Total Volume	40	0	0	0	40	159	1308	0	0	1467	49	0	0	0	49	38	2329	0	0	2367	3923	
% App. Total	PHF	.625	.000	.000	.000	.625	.811	.929	.000	.000	.915	.817	.000	.000	.000	.817	.792	.987	.000	.000	.985	.959



PRECISION
DATA
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N/S: Blossom Street/ Blossom Street Ext
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 BB
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Heavy Vehicles

	Shepard Street From North				Lynnway (Route 1A) From East				Marine Boulevard From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
04:00 PM	0	0	0	0	2	18	0	0	0	0	0	0	0	3	0	0	23
04:15 PM	0	0	0	0	2	12	0	0	1	0	0	0	0	12	0	0	27
04:30 PM	0	0	0	0	3	13	0	0	0	0	0	0	0	8	0	0	24
04:45 PM	1	0	0	0	1	5	0	0	0	0	0	0	0	5	0	0	12
Total	1	0	0	0	8	48	0	0	1	0	0	0	0	28	0	0	86
05:00 PM	1	0	0	0	3	8	0	0	0	0	0	0	0	9	0	0	21
05:15 PM	0	0	0	0	2	9	0	0	1	0	0	0	0	4	0	0	16
05:30 PM	0	0	0	0	0	9	0	0	0	0	0	0	0	12	0	0	21
05:45 PM	0	0	0	0	0	2	0	0	0	0	0	0	0	9	0	0	11
Total	1	0	0	0	5	28	0	0	1	0	0	0	0	34	0	0	69
Grand Total	2	0	0	0	13	76	0	0	2	0	0	0	0	62	0	0	155
Apprch %	100	0	0	0	14.6	85.4	0	0	100	0	0	0	0	100	0	0	
Total %	1.3	0	0	0	8.4	49	0	0	1.3	0	0	0	0	40	0	0	

	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					Int. Total
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	
Start Time																					
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:00 PM																					
04:00 PM	0	0	0	0	0	2	18	0	0	20	0	0	0	0	0	0	3	0	0	3	23
04:15 PM	0	0	0	0	0	2	12	0	0	14	1	0	0	0	1	0	12	0	0	12	27
04:30 PM	0	0	0	0	0	3	13	0	0	16	0	0	0	0	0	0	8	0	0	8	24
04:45 PM	1	0	0	0	1	1	5	0	0	6	0	0	0	0	0	0	5	0	0	5	12
Total Volume	1	0	0	0	1	8	48	0	0	56	1	0	0	0	1	0	28	0	0	28	86
% App. Total	100	0	0	0		14.3	85.7	0	0		100	0	0	0		0	100	0	0		
PHF	.250	.000	.000	.000	.250	.667	.667	.000	.000	.700	.250	.000	.000	.000	.250	.000	.583	.000	.000	.583	.796



PRECISION
DATA
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N/S: Blossom Street/ Blossom Street Ext
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 BB
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Peds and Bikes

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	Peds EB	Peds WB	Right	Thru	Left	Peds SB	Peds NB	Right	Thru	Left	Peds WB	Peds EB	Right	Thru	Left	Peds NB	Peds SB	Int. Total
04:00 PM	0	0	0	2	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
04:15 PM	0	0	0	6	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10
04:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
04:45 PM	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
Total	0	0	0	13	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18
05:00 PM	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
05:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
05:30 PM	0	0	0	2	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	0	4
05:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
Total	0	0	0	3	2	0	0	0	0	1	1	0	0	0	0	0	0	0	1	0	8
Grand Total	0	0	0	16	7	0	0	0	1	1	0	0	0	0	0	0	0	0	1	0	26
Apprch %	0	0	0	69.6	30.4	0	0	0	50	50	0	0	0	0	0	0	0	0	100	0	
Total %	0	0	0	61.5	26.9	0	0	0	3.8	3.8	0	0	0	0	0	0	0	0	3.8	0	

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West								
	Right	Thru	Left	Peds EB	Peds WB	App. Total	Right	Thru	Left	Peds SB	Peds NB	App. Total	Right	Thru	Left	Peds WB	Peds EB	App. Total	Right	Thru	Left	Peds NB	Peds SB	App. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																								
Peak Hour for Entire Intersection Begins at 04:00 PM																								
04:00 PM	0	0	0	2	1	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	6	4	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10
04:30 PM	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
04:45 PM	0	0	0	4	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
Total Volume	0	0	0	13	5	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18
% App. Total	0	0	0	72.2	27.8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
PHF	.000	.000	.000	.542	.313	.450	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.450



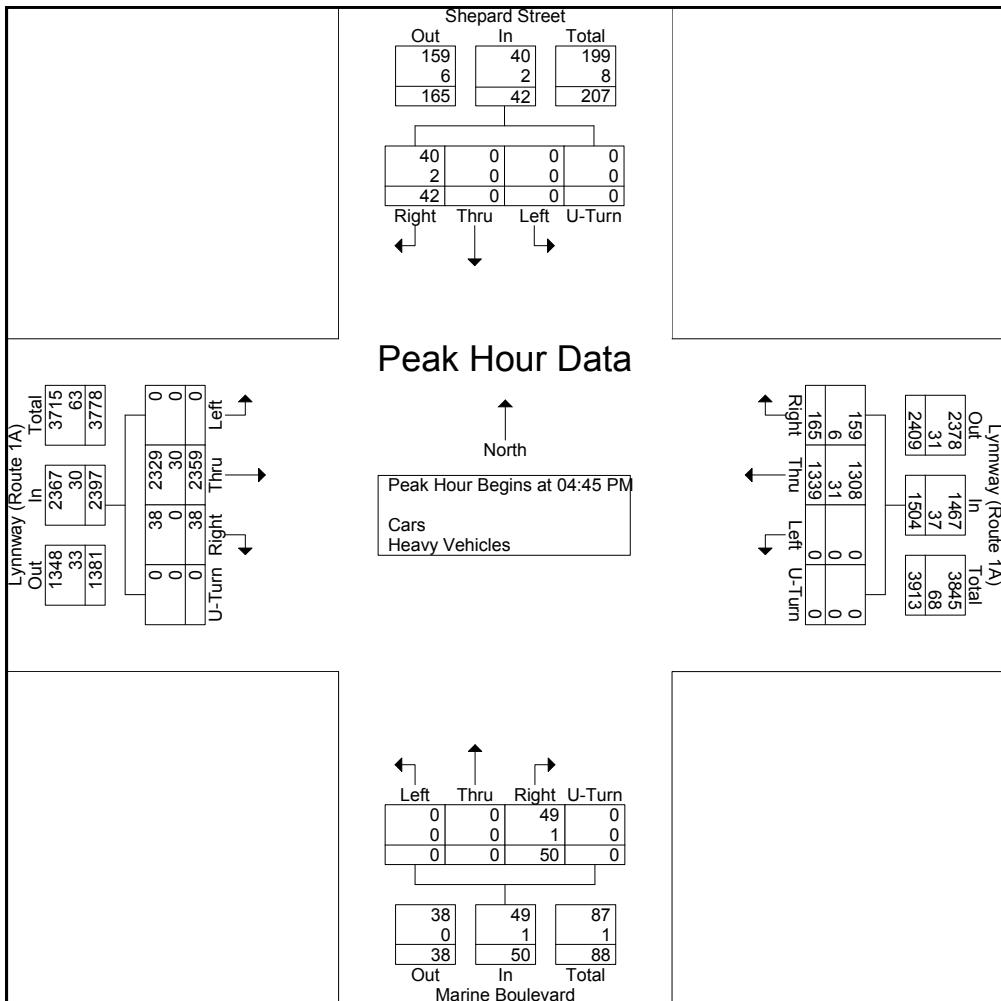
PRECISION
D A T A
INDUSTRIES, LLC

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N/S: Blossom Street/ Blossom Street Ext
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 BB
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Start Time	Shepard Street From North					Lynnway (Route 1A) From East					Marine Boulevard From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
04:45 PM	7	0	0	0	7	35	325	0	0	360	15	0	0	0	15	12	579	0	0	591	973
05:00 PM	14	0	0	0	14	52	360	0	0	412	14	0	0	0	14	9	595	0	0	604	1044
05:15 PM	5	0	0	0	5	40	340	0	0	380	12	0	0	0	12	11	594	0	0	605	1002
05:30 PM	16	0	0	0	16	38	314	0	0	352	9	0	0	0	9	6	591	0	0	597	974
Total Volume % App. Total	42	0	0	0	42	165	1339	0	0	1504	50	0	0	0	50	38	2359	0	0	2397	3993
PHF	.656	.000	.000	.000	.656	.793	.930	.000	.000	.913	.833	.000	.000	.000	.833	.792	.991	.000	.000	.990	.956
Cars	40	0	0	0	40	159	1308	0	0	1467	49	0	0	0	49	38	2329	0	0	2367	3923
% Cars	95.2	0	0	0	95.2	96.4	97.7	0	0	97.5	98.0	0	0	0	98.0	100	98.7	0	0	98.7	98.2
Heavy Vehicles % Heavy Vehicles	4.8	0	0	0	4.8	3.6	2.3	0	0	2.5	2.0	0	0	0	2.0	0	1.3	0	0	1.3	1.8





N/S: Jughandle/Kingman Street
 E/W: Lynnway (Route 1A)
 City, State: Lynn, MA
 Client: VHB/K. Keen

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File Name : 144179 C
 Site Code : 11942.03
 Start Date : 11/18/2014
 Page No : 1

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West				
Start Time	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Int. Total
07:00 AM	0	0	0	0	41	731	11	0	6	0	8	0	9	199	3	2	1010
07:15 AM	0	0	0	0	44	725	12	0	11	2	11	0	8	218	5	1	1037
07:30 AM	0	0	0	0	41	632	17	3	9	1	6	0	6	275	3	9	1002
07:45 AM	0	0	0	0	40	636	18	2	9	0	6	0	11	290	6	5	1023
Total	0	0	0	0	166	2724	58	5	35	3	31	0	34	982	17	17	4072
08:00 AM	0	0	0	0	44	542	20	5	5	0	14	0	17	241	5	13	906
08:15 AM	0	0	0	0	39	568	21	5	16	0	13	0	16	237	5	2	922
08:30 AM	0	0	0	0	41	474	26	0	5	1	10	0	30	257	12	8	864
08:45 AM	1	0	1	0	33	447	30	3	7	0	8	0	14	247	5	5	801
Total	1	0	1	0	157	2031	97	13	33	1	45	0	77	982	27	28	3493
Grand Total	1	0	1	0	323	4755	155	18	68	4	76	0	111	1964	44	45	7565
Apprch %	50	0	50	0	6.2	90.6	3	0.3	45.9	2.7	51.4	0	5.1	90.8	2	2.1	
Total %	0	0	0	0	4.3	62.9	2	0.2	0.9	0.1	1	0	1.5	26	0.6	0.6	
Cars	1	0	1	0	314	4682	146	18	37	2	43	0	101	1849	42	44	7280
% Cars	100	0	100	0	97.2	98.5	94.2	100	54.4	50	56.6	0	91	94.1	95.5	97.8	96.2
Heavy Vehicles	0	0	0	0	9	73	9	0	31	2	33	0	10	115	2	1	285
% Heavy Vehicles	0	0	0	0	2.8	1.5	5.8	0	45.6	50	43.4	0	9	5.9	4.5	2.2	3.8

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West									
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																						
Peak Hour for Entire Intersection Begins at 07:00 AM																						
07:00 AM	0	0	0	0	0	41	731	11	0	783	6	0	8	0	14	9	199	3	2	213	1010	
07:15 AM	0	0	0	0	0	44	725	12	0	781	11	2	11	0	24	8	218	5	1	232	1037	
07:30 AM	0	0	0	0	0	41	632	17	3	693	9	1	6	0	16	6	275	3	9	293	1002	
07:45 AM	0	0	0	0	0	40	636	18	2	696	9	0	6	0	15	11	290	6	5	312	1023	
Total Volume	0	0	0	0	0	166	2724	58	5	2953	35	3	31	0	69	34	982	17	17	1050	4072	
% App. Total	.PHF	.000	.000	.000	.000	.000	.943	.932	.806	.417	.943	.795	.375	.705	.000	.719	.773	.847	.708	.472	.841	.982
Cars	0	0	0	0	0	163	2696	54	5	2918	20	1	12	0	33	29	917	15	16	977	3928	
% Cars	0	0	0	0	0	98.2	99.0	93.1	100	98.8	57.1	33.3	38.7	0	47.8	85.3	93.4	88.2	94.1	93.0	96.5	
Heavy Vehicles	0	0	0	0	0	1.8	1.0	6.9	0	1.2	42.9	66.7	61.3	0	52.2	14.7	6.6	11.8	5.9	7.0	3.5	



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N/S: Jughandle/Kingman Street
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 C
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Cars

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
07:00 AM	0	0	0	0	39	721	9	0	3	0	3	0	8	183	1	2	969
07:15 AM	0	0	0	0	43	719	11	0	8	1	3	0	7	210	5	1	1008
07:30 AM	0	0	0	0	41	626	16	3	5	0	2	0	6	250	3	9	961
07:45 AM	0	0	0	0	40	630	18	2	4	0	4	0	8	274	6	4	990
Total	0	0	0	0	163	2696	54	5	20	1	12	0	29	917	15	16	3928
08:00 AM	0	0	0	0	44	531	19	5	2	0	13	0	15	230	5	13	877
08:15 AM	0	0	0	0	37	557	20	5	5	0	6	0	15	227	5	2	879
08:30 AM	0	0	0	0	39	461	25	0	3	1	6	0	29	240	12	8	824
08:45 AM	1	0	1	0	31	437	28	3	7	0	6	0	13	235	5	5	772
Total	1	0	1	0	151	1986	92	13	17	1	31	0	72	932	27	28	3352
Grand Total	1	0	1	0	314	4682	146	18	37	2	43	0	101	1849	42	44	7280
Apprch %	50	0	50	0	6.1	90.7	2.8	0.3	45.1	2.4	52.4	0	5	90.8	2.1	2.2	
Total %	0	0	0	0	4.3	64.3	2	0.2	0.5	0	0.6	0	1.4	25.4	0.6	0.6	

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West				Int. Total				
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn		
Start Time																					
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:00 AM																					
07:00 AM	0	0	0	0	0	39	721	9	0	769	3	0	3	0	6	8	183	1	2	194	
07:15 AM	0	0	0	0	0	43	719	11	0	773	8	1	3	0	12	7	210	5	1	223	
07:30 AM	0	0	0	0	0	41	626	16	3	686	5	0	2	0	7	6	250	3	9	268	
07:45 AM	0	0	0	0	0	40	630	18	2	690	4	0	4	0	8	8	274	6	4	292	
Total Volume	0	0	0	0	0	163	2696	54	5	2918	20	1	12	0	33	29	917	15	16	977	
% App. Total																					
PHF	.000	.000	.000	.000	.000	.948	.935	.750	.417	.944	.625	.250	.750	.000	.688	.906	.837	.625	.444	.836	.974



N/S: Jughandle/Kingman Street
 E/W: Lynnway (Route 1A)
 City, State: Lynn, MA
 Client: VHB/K. Keen

P.O. Box 301 Berlin, MA 01503
 Office: 508.481.3999 Fax: 508.545.1234
 Email: datarequests@pdillc.com

File Name : 144179 C
 Site Code : 11942.03
 Start Date : 11/18/2014
 Page No : 1

Groups Printed- Heavy Vehicles

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
07:00 AM	0	0	0	0	2	10	2	0	3	0	5	0	1	16	2	0	41
07:15 AM	0	0	0	0	1	6	1	0	3	1	8	0	1	8	0	0	29
07:30 AM	0	0	0	0	0	6	1	0	4	1	4	0	0	25	0	0	41
07:45 AM	0	0	0	0	0	6	0	0	5	0	2	0	3	16	0	1	33
Total	0	0	0	0	3	28	4	0	15	2	19	0	5	65	2	1	144
08:00 AM	0	0	0	0	0	11	1	0	3	0	1	0	2	11	0	0	29
08:15 AM	0	0	0	0	2	11	1	0	11	0	7	0	1	10	0	0	43
08:30 AM	0	0	0	0	2	13	1	0	2	0	4	0	1	17	0	0	40
08:45 AM	0	0	0	0	2	10	2	0	0	0	2	0	1	12	0	0	29
Total	0	0	0	0	6	45	5	0	16	0	14	0	5	50	0	0	141
Grand Total	0	0	0	0	9	73	9	0	31	2	33	0	10	115	2	1	285
Apprch %	0	0	0	0	9.9	80.2	9.9	0	47	3	50	0	7.8	89.8	1.6	0.8	
Total %	0	0	0	0	3.2	25.6	3.2	0	10.9	0.7	11.6	0	3.5	40.4	0.7	0.4	

	Jughandle From North					Lynnway (Route 1A) From East					Kingman Street From South					Lynnway (Route 1A) From West					Int. Total
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																					
Start Time																					
07:30 AM	0	0	0	0	0	0	6	1	0	7	4	1	4	0	9	0	25	0	0	25	41
07:45 AM	0	0	0	0	0	0	6	0	0	6	5	0	2	0	7	3	16	0	1	20	33
08:00 AM	0	0	0	0	0	0	11	1	0	12	3	0	1	0	4	2	11	0	0	13	29
08:15 AM	0	0	0	0	0	2	11	1	0	14	11	0	7	0	18	1	10	0	0	11	43
Total Volume	0	0	0	0	0	2	34	3	0	39	23	1	14	0	38	6	62	0	1	69	146
% App. Total	0	0	0	0		5.1	87.2	7.7	0		60.5	2.6	36.8	0		8.7	89.9	0	1.4		
PHF	.000	.000	.000	.000	.000	.250	.773	.750	.000	.696	.523	.250	.500	.000	.528	.500	.620	.000	.250	.690	.849



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N/S: Jughandle/Kingman Street
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 C
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Peds and Bikes

Start Time	Jughandle From North					Lynnway (Route 1A) From East					Kingman Street From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	Peds EB	Peds WB	Right	Thru	Left	Peds SB	Peds NB	Right	Thru	Left	Peds WB	Peds EB	Right	Thru	Left	Peds NB	Peds SB	Int. Total
07:00 AM	0	0	0	2	2	0	0	0	2	0	0	0	0	2	0	0	0	0	0	0	8
07:15 AM	0	0	0	2	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	4
07:30 AM	0	0	0	0	2	0	0	0	1	1	0	0	0	1	0	0	0	0	0	0	5
07:45 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	3
Total	0	0	0	4	5	0	0	0	3	1	0	0	0	6	0	0	0	0	1	0	20
08:00 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	0	0	0	1	0	3
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
08:30 AM	0	0	0	1	0	0	0	0	2	0	0	0	0	3	0	0	0	0	0	0	6
08:45 AM	0	0	0	0	1	0	0	0	1	1	0	0	0	0	1	0	0	0	0	0	4
Total	0	0	0	1	1	0	0	0	4	1	0	0	0	5	2	0	0	0	1	0	15
Grand Total	0	0	0	5	6	0	0	0	7	2	0	0	0	11	2	0	0	0	2	0	35
Apprch %	0	0	0	45.5	54.5	0	0	0	77.8	22.2	0	0	0	84.6	15.4	0	0	0	100	0	
Total %	0	0	0	14.3	17.1	0	0	0	20	5.7	0	0	0	31.4	5.7	0	0	0	5.7	0	

Start Time	Jughandle From North					Lynnway (Route 1A) From East					Kingman Street From South					Lynnway (Route 1A) From West				
	Right	Thru	Left	Peds EB	Peds WB	Right	Thru	Left	Peds SB	Peds NB	Right	Thru	Left	Peds WB	Peds EB	Right	Thru	Left	Peds NB	Peds SB

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

07:00 AM	0	0	0	2	2	4	0	0	0	2	0	2	0	0	0	2	0	0	0	0	8	
07:15 AM	0	0	0	2	0	2	0	0	0	0	0	0	0	1	0	1	0	0	1	0	4	
07:30 AM	0	0	0	0	2	2	0	0	0	1	1	2	0	0	0	1	0	0	0	0	5	
07:45 AM	0	0	0	0	1	1	0	0	0	0	0	0	0	2	0	2	0	0	0	0	3	
Total Volume	0	0	0	4	5	9	0	0	0	3	1	4	0	0	0	6	0	0	0	1	20	
% App. Total	0	0	0	44.4	55.6		0	0	0	75	25		0	0	0	100	0	0	0	100	0	
PHF	.000	.000	.000	.500	.625	.563	.000	.000	.000	.375	.250	.500	.000	.000	.000	.750	.000	.000	.250	.000	.625	



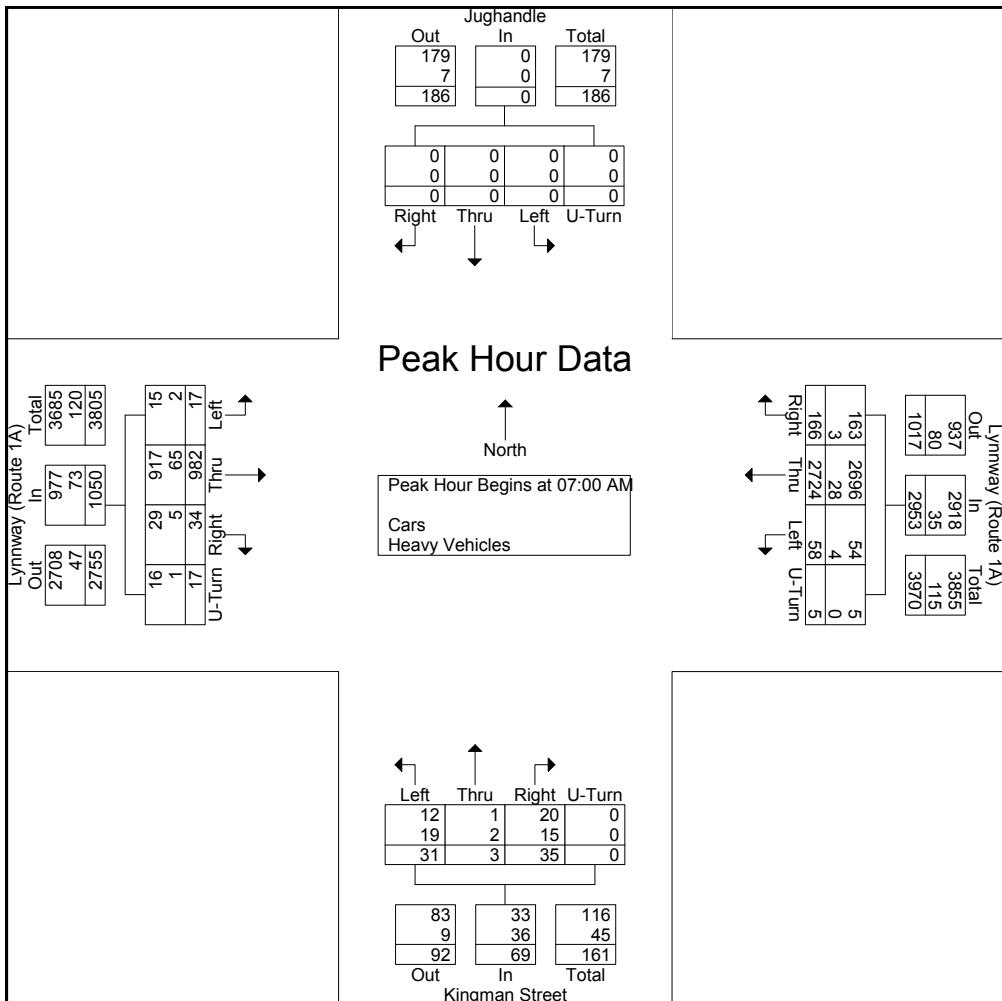
**PRECISION
D A T A
INDUSTRIES, LLC**

P.O. Box 301 Berlin, MA 01503
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Email: datarequests@pdill.com

N/S: Jughandle/Kingman Street
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 C
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Start Time	Jughandle From North					Lynnway (Route 1A) From East					Kingman Street From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																					
07:00 AM	0	0	0	0	0	41	731	11	0	783	6	0	8	0	14	9	199	3	2	213	1010
07:15 AM	0	0	0	0	0	44	725	12	0	781	11	2	11	0	24	8	218	5	1	232	1037
07:30 AM	0	0	0	0	0	41	632	17	3	693	9	1	6	0	16	6	275	3	9	293	1002
07:45 AM	0	0	0	0	0	40	636	18	2	696	9	0	6	0	15	11	290	6	5	312	1023
Total Volume % App. Total	0	0	0	0	0	166	2724	58	5	2953	35	3	31	0	69	34	982	17	17	1050	4072
PHF	.000	.000	.000	.000	.000	.943	.932	.806	.417	.943	.795	.375	.705	.000	.719	.773	.847	.708	.472	.841	.982
Cars	0	0	0	0	0	163	2696	54	5	2918	20	1	12	0	33	29	917	15	16	977	3928
% Cars	0	0	0	0	0	98.2	99.0	93.1	100	98.8	57.1	33.3	38.7	0	47.8	85.3	93.4	88.2	94.1	93.0	96.5
Heavy Vehicles	0	0	0	0	0	1.8	1.0	6.9	0	1.2	42.9	66.7	61.3	0	52.2	14.7	6.6	11.8	5.9	7.0	3.5





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N/S: Jughandle/Kingman Street
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 CC
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Cars - Heavy Vehicles

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West				Int. Total
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	
Start Time																	
04:00 PM	0	0	0	0	9	314	14	6	23	0	34	0	3	439	2	7	851
04:15 PM	0	0	0	0	13	297	7	5	11	0	24	0	7	467	1	8	840
04:30 PM	0	1	0	0	4	303	8	3	21	1	16	0	11	534	12	11	925
04:45 PM	1	0	0	0	12	315	19	5	20	0	25	0	12	549	3	11	972
Total	1	1	0	0	38	1229	48	19	75	1	99	0	33	1989	18	37	3588
05:00 PM	0	0	0	0	7	321	13	4	47	0	59	0	2	564	2	20	1039
05:15 PM	0	0	0	0	8	345	10	4	17	1	33	0	3	609	3	10	1043
05:30 PM	1	0	0	0	6	291	14	2	14	0	24	0	5	587	2	7	953
05:45 PM	0	0	0	0	7	255	17	0	14	0	14	0	5	622	3	9	946
Total	1	0	0	0	28	1212	54	10	92	1	130	0	15	2382	10	46	3981
Grand Total	2	1	0	0	66	2441	102	29	167	2	229	0	48	4371	28	83	7569
Apprch %	66.7	33.3	0	0	2.5	92.5	3.9	1.1	42	0.5	57.5	0	1.1	96.5	0.6	1.8	
Total %	0	0	0	0	0.9	32.2	1.3	0.4	2.2	0	3	0	0.6	57.7	0.4	1.1	
Cars	2	1	0	0	65	2377	74	29	162	1	214	0	37	4321	25	83	7391
% Cars	100	100	0	0	98.5	97.4	72.5	100	97	50	93.4	0	77.1	98.9	89.3	100	97.6
Heavy Vehicles	0	0	0	0	1	64	28	0	5	1	15	0	11	50	3	0	178
% Heavy Vehicles	0	0	0	0	1.5	2.6	27.5	0	3	50	6.6	0	22.9	1.1	10.7	0	2.4

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West				Int. Total				
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total					
Start Time																					
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:45 PM																					
04:45 PM	1	0	0	0	1	12	315	19	5	351	20	0	25	0	45	12	549	3	11	575	972
05:00 PM	0	0	0	0	0	7	321	13	4	345	47	0	59	0	106	2	564	2	20	588	1039
05:15 PM	0	0	0	0	0	8	345	10	4	367	17	1	33	0	51	3	609	3	10	625	1043
05:30 PM	1	0	0	0	1	6	291	14	2	313	14	0	24	0	38	5	587	2	7	601	953
Total Volume	2	0	0	0	2	33	1272	56	15	1376	98	1	141	0	240	22	2309	10	48	2389	4007
% App. Total	.500	.000	.000	.000	.500	.688	.922	.737	.750	.937	.521	.250	.597	.000	.566	.458	.948	.833	.600	.956	.960
PHF	2	0	0	0	2	32	1241	42	15	1330	95	0	135	0	230	16	2286	10	48	2360	3922
Cars	100	0	0	0	100	97.0	97.6	75.0	100	96.7	96.9	0	95.7	0	95.8	72.7	99.0	100	100	98.8	97.9
Heavy Vehicles	0	0	0	0	0	3.0	2.4	25.0	0	3.3	3.1	100	4.3	0	4.2	27.3	1.0	0	0	1.2	2.1



N/S: Jughandle/Kingman Street
 E/W: Lynnway (Route 1A)
 City, State: Lynn, MA
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File Name : 144179 CC
 Site Code : 11942.03
 Start Date : 11/18/2014
 Page No : 1

Groups Printed- Cars

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West				
Start Time	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Int. Total
04:00 PM	0	0	0	0	9	304	11	6	23	0	30	0	3	434	2	7	829
04:15 PM	0	0	0	0	13	285	7	5	11	0	21	0	5	457	0	8	812
04:30 PM	0	1	0	0	4	294	5	3	20	1	14	0	10	529	10	11	902
04:45 PM	1	0	0	0	12	309	14	5	20	0	25	0	11	542	3	11	953
Total	1	1	0	0	38	1192	37	19	74	1	90	0	29	1962	15	37	3496
05:00 PM	0	0	0	0	6	313	11	4	46	0	55	0	1	559	2	20	1017
05:15 PM	0	0	0	0	8	336	9	4	15	0	32	0	1	605	3	10	1023
05:30 PM	1	0	0	0	6	283	8	2	14	0	23	0	3	580	2	7	929
05:45 PM	0	0	0	0	7	253	9	0	13	0	14	0	3	615	3	9	926
Total	1	0	0	0	27	1185	37	10	88	0	124	0	8	2359	10	46	3895
Grand Total	2	1	0	0	65	2377	74	29	162	1	214	0	37	4321	25	83	7391
Apprch %	66.7	33.3	0	0	2.6	93.4	2.9	1.1	43	0.3	56.8	0	0.8	96.8	0.6	1.9	
Total %	0	0	0	0	0.9	32.2	1	0.4	2.2	0	2.9	0	0.5	58.5	0.3	1.1	

	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West									
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																						
Peak Hour for Entire Intersection Begins at 04:45 PM																						
04:45 PM	1	0	0	0	1	12	309	14	5	340	20	0	25	0	45	11	542	3	11	567	953	
05:00 PM	0	0	0	0	0	6	313	11	4	334	46	0	55	0	101	1	559	2	20	582	1017	
05:15 PM	0	0	0	0	0	8	336	9	4	357	15	0	32	0	47	1	605	3	10	619	1023	
05:30 PM	1	0	0	0	1	6	283	8	2	299	14	0	23	0	37	3	580	2	7	592	929	
Total Volume	2	0	0	0	2	32	1241	42	15	1330	95	0	135	0	230	16	2286	10	48	2360	3922	
% App. Total	PHF	.500	.000	.000	.000	.500	.667	.923	.750	.750	.931	.516	.000	.614	.000	.569	.364	.945	.833	.600	.953	.958



N/S: Jughandle/Kingman Street
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	Jughandle From North				Lynnway (Route 1A) From East				Kingman Street From South				Lynnway (Route 1A) From West				
Start Time	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Int. Total
04:00 PM	0	0	0	0	0	10	3	0	0	0	4	0	0	5	0	0	22
04:15 PM	0	0	0	0	0	12	0	0	0	0	3	0	2	10	1	0	28
04:30 PM	0	0	0	0	0	9	3	0	1	0	2	0	1	5	2	0	23
04:45 PM	0	0	0	0	0	6	5	0	0	0	0	0	1	7	0	0	19
Total	0	0	0	0	0	37	11	0	1	0	9	0	4	27	3	0	92
05:00 PM	0	0	0	0	1	8	2	0	1	0	4	0	1	5	0	0	22
05:15 PM	0	0	0	0	0	9	1	0	2	1	1	0	2	4	0	0	20
05:30 PM	0	0	0	0	0	8	6	0	0	0	1	0	2	7	0	0	24
05:45 PM	0	0	0	0	0	2	8	0	1	0	0	0	2	7	0	0	20
Total	0	0	0	0	1	27	17	0	4	1	6	0	7	23	0	0	86
Grand Total	0	0	0	0	1	64	28	0	5	1	15	0	11	50	3	0	178
Apprch %	0	0	0	0	1.1	68.8	30.1	0	23.8	4.8	71.4	0	17.2	78.1	4.7	0	
Total %	0	0	0	0	0.6	36	15.7	0	2.8	0.6	8.4	0	6.2	28.1	1.7	0	

	Jughandle From North					Lynnway (Route 1A) From East					Kingman Street From South					Lynnway (Route 1A) From West					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
04:00 PM	0	0	0	0	0	0	10	3	0	13	0	0	4	0	4	0	5	0	0	5	22
04:15 PM	0	0	0	0	0	0	12	0	0	12	0	0	3	0	3	2	10	1	0	13	28
04:30 PM	0	0	0	0	0	0	9	3	0	12	1	0	2	0	3	1	5	2	0	8	23
04:45 PM	0	0	0	0	0	0	6	5	0	11	0	0	0	0	0	1	7	0	0	8	19
Total Volume	0	0	0	0	0	0	37	11	0	48	1	0	9	0	10	4	27	3	0	34	92
% App. Total	0	0	0	0	0	0	77.1	22.9	0	10	0	90	0	10	11.8	79.4	8.8	0	0	0	92
PHF	.000	.000	.000	.000	.000	.000	.771	.550	.000	.923	.250	.000	.563	.000	.625	.500	.675	.375	.000	.654	.821



PRECISION
D A T A
INDUSTRIES,LLC

P.O.Box 301 Berlin, MA 01503
Office: 508.481.3999 Fax: 508.545.1234
Email: datarequests@pdill.com

N/S: Jughandle/Kingman Street
E/W: Lynnway (Route 1A)
City, State: Lynn, MA
Client: VHB/K. Keen

File Name : 144179 CC
Site Code : 11942.03
Start Date : 11/18/2014
Page No : 1

Groups Printed- Peds and Bikes

Start Time	Jughandle From North					Lynnway (Route 1A) From East					Kingman Street From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	Peds EB	Peds WB	Right	Thru	Left	Peds SB	Peds NB	Right	Thru	Left	Peds WB	Peds EB	Right	Thru	Left	Peds NB	Peds SB	Int. Total
04:00 PM	0	0	0	2	4	0	0	0	1	0	0	0	0	0	1	0	0	0	0	0	8
04:15 PM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
04:30 PM	0	0	0	0	2	0	0	0	0	2	0	0	0	0	2	0	0	0	0	0	6
04:45 PM	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	2
Total	0	0	0	5	6	0	0	0	2	2	0	0	0	0	3	0	0	0	0	0	18
05:00 PM	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
05:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	2
05:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	2
05:45 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	1	0	0	0	0	1	0	5
Total	0	0	0	0	0	0	0	0	1	6	0	0	0	1	1	0	0	0	2	0	11
Grand Total	0	0	0	5	6	0	0	0	3	8	0	0	0	1	4	0	0	0	2	0	29
Apprch %	0	0	0	45.5	54.5	0	0	0	27.3	72.7	0	0	0	20	80	0	0	0	100	0	0
Total %	0	0	0	17.2	20.7	0	0	0	10.3	27.6	0	0	0	3.4	13.8	0	0	0	6.9	0	0

Start Time	Jughandle From North					Lynnway (Route 1A) From East					Kingman Street From South					Lynnway (Route 1A) From West					Int. Total	
	Right	Thru	Left	Peds EB	Peds WB	Right	Thru	Left	Peds SB	Peds NB	Right	Thru	Left	Peds WB	Peds EB	Right	Thru	Left	Peds NB	Peds SB	Int. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																						
Peak Hour for Entire Intersection Begins at 04:00 PM																						
04:00 PM	0	0	0	2	4	6	0	0	0	1	0	1	0	0	1	1	0	0	0	0	0	
04:15 PM	0	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
04:30 PM	0	0	0	0	2	2	0	0	0	0	2	2	0	0	0	2	0	0	0	0	6	
04:45 PM	0	0	0	1	0	1	0	0	0	1	0	1	0	0	0	0	0	0	0	0	2	
Total Volume	0	0	0	5	6	11	0	0	0	2	2	4	0	0	0	3	3	0	0	0	18	
% App. Total	0	0	0	45.5	54.5		0	0	0	50	50		0	0	0	100		0	0	0	0	
PHF	.000	.000	.000	.625	.375	.458	.000	.000	.000	.500	.250	.500	.000	.000	.000	.375	.375	.000	.000	.000	.563	



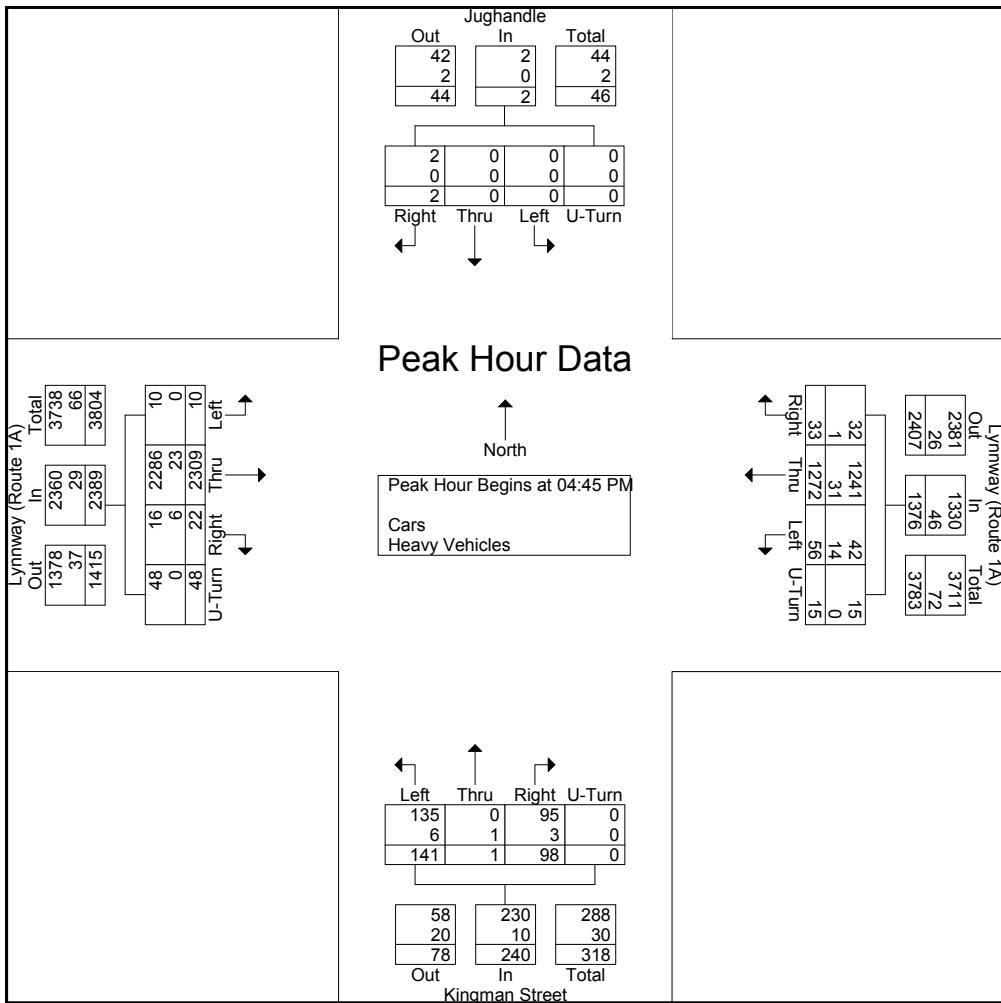
PRECISION
D A T A
INDUSTRIES, LLC

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N/S: Jughandle/Kingman Street
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Start Time	Jughandle From North					Lynnway (Route 1A) From East					Kingman Street From South					Lynnway (Route 1A) From West					
	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																					
04:45 PM	1	0	0	0	1	12	315	19	5	351	20	0	25	0	45	12	549	3	11	575	972
05:00 PM	0	0	0	0	0	7	321	13	4	345	47	0	59	0	106	2	564	2	20	588	1039
05:15 PM	0	0	0	0	0	8	345	10	4	367	17	1	33	0	51	3	609	3	10	625	1043
05:30 PM	1	0	0	0	1	6	291	14	2	313	14	0	24	0	38	5	587	2	7	601	953
Total Volume % App. Total	2	0	0	0	2	33	1272	56	15	1376	98	1	141	0	240	22	2309	10	48	2389	4007
PHF	.500	.000	.000	.000	.500	.688	.922	.737	.750	.937	.521	.250	.597	.000	.566	.458	.948	.833	.600	.956	.960
Cars	2	0	0	0	2	32	1241	42	15	1330	95	0	135	0	230	16	2286	10	48	2360	3922
% Cars	100	0	0	0	100	97.0	97.6	75.0	100	96.7	96.9	0	95.7	0	95.8	72.7	99.0	100	100	98.8	97.9
Heavy Vehicles	0	0	0	0	0	3.0	2.4	25.0	0	3.3	3.1	100	4.3	0	4.2	27.3	1.0	0	0	1.2	2.1



2014 Lynn to Boston Ferry Schedule

Lynn to Boston Ferry Schedule

Monday through Friday Service

May 19, 2014 through September 12, 2014

Inbound

Depart Lynn (Blossom Street)	Arrive Boston (Central Wharf)
6:30 am	7:05 am
8:00 am	8:35 am
6:30 pm	7:05 pm

Outbound

Depart Boston (Central Wharf)	Arrive Lynn (Blossom Street)
7:15 am	7:45 am
5:45 pm	6:20 pm
7:15 pm	7:50 pm

Rates

One way: \$7.00

Children (3-12) and Seniors: \$3.50

Children under three years of age and under: FREE

MBTA Zone 2 pass or higher



Lynn Commuter Ferry Ridership Data

BOSTON HARBOR CRUISES
RIDERSHIP BY DAY OF THE WEEK BY TIME

DEPARTING	LYNN	September-14												TOTAL														
		mon	tue	wed	thu	fri	sat	sun	tue	wed	thu	fri	sat															
06:30 AM	18	24	23	16	15	15	18	17	18	19	15	13	19	15	12	15	9	15	312									
08:00 AM	48	51	54	49	45	48	40	37	47	48	37	42	35	56	43	37	47	36	41	841								
06:30 PM	0	2	5	8	9	4	2	7	13	0	0	0	0	3	0	3	0	6	0	7	66							
Total Lynn Departures	0	66	77	82	73	0	69	67	60	61	78	0	64	56	57	48	78	0	58	55	62	45	63	0	0	0	0	1,219

DEPARTING	BOSTON	September-14												TOTAL													
		mon	tue	wed	thu	fri	sat	sun	tue	wed	thu	fri	sat														
07:15 AM	4	4	0	3	3	3	1	0	1	0	1	1	0	2	1	1	1	3	30								
05:45 PM	64	54	57	57	49	47	40	49	60	41	41	46	35	61	52	49	43	39	41	925							
07:15 PM	9	22	20	18	30	10	13	13	21	13	12	10	14	6	6	10	8	3	17	255							
Total Boston Departures	0	77	80	77	78	0	82	60	54	62	82	0	54	57	49	69	0	59	53	43	61	0	0	0	0	1,210	
TOTAL RIDERS	0	143	157	159	151	0	151	127	114	123	160	0	118	110	114	97	147	0	117	114	115	88	124	0	0	0	2,429

Subtotal A.M.
Subtotal P.M.

Weekly Ridership

END DATE EXTENDED PERIOD

Cumulative ridership from inception

13,136

Intersection Capacity Analyses

11942.03 :: Blossom Street Improvements

1: Marine Boulevard/Shepard Street & Lynnway (Route 1A)

2014 Existing Conditions with Ferry Traffic

Timing Plan: Weekday Morning

Lane Group	EBU	EBL	EBT	EBC	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↑↑↑				↑↑↑			↔			↔	
Volume (vph)	5	35	1040	15	75	5	2395	50	10	10	10	25	0	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		0		350		0	0		0	0	0	0	0
Storage Lanes	1		0		1		0	0		0	0	0	0	0
Taper Length (ft)	25				25			25				25		
Satd. Flow (prot)	0	1687	4836	0	0	1770	5067	0	0	1056	0	0	1328	0
Flt Permitted		0.950				0.950				0.871			0.802	
Satd. Flow (perm)	0	1686	4836	0	0	1767	5067	0	0	934	0	0	1094	0
Right Turn on Red				Yes				Yes			Yes			Yes
Satd. Flow (RTOR)		3					4			15			65	
Link Speed (mph)		30					30			30			30	
Link Distance (ft)		520					631			362			232	
Travel Time (s)		11.8					14.3			8.2			5.3	
Conf. Peds. (#/hr)		8		3		3		8	2				2	
Peak Hour Factor	0.83	0.83	0.83	0.83	0.94	0.94	0.94	0.94	0.65	0.65	0.65	1.00	1.00	1.00
Heavy Vehicles (%)	7%	7%	7%	7%	2%	2%	2%	2%	69%	69%	69%	30%	30%	30%
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	48	1271	0	0	85	2601	0	0	45	0	0	45	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	1	1	6		5	5	2			4			4	
Permitted Phases														
Detector Phase	1	1	6		5	5	2		4	4		4	4	
Switch Phase														
Minimum Initial (s)	7.0	7.0	15.0		7.0	7.0	15.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	12.0	12.0	20.0		12.0	12.0	20.0		27.0	27.0		27.0	27.0	
Total Split (s)	25.0	25.0	65.0		25.0	25.0	65.0		27.0	27.0		27.0	27.0	
Total Split (%)	21.4%	21.4%	55.6%		21.4%	21.4%	55.6%		23.1%	23.1%		23.1%	23.1%	
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		4.0	4.0		4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0			0.0	0.0			0.0	0.0		0.0		
Total Lost Time (s)		5.0	5.0			5.0	5.0			6.0			6.0	
Lead/Lag	Lead	Lead	Lag		Lead	Lead	Lag							
Lead-Lag Optimize?														
Recall Mode	None	None	Max		None	None	Max		None	None		None	None	
Act Effct Green (s)		8.0	65.4			9.3	69.2			10.0			10.0	
Actuated g/C Ratio		0.09	0.70			0.10	0.74			0.11			0.11	
v/c Ratio		0.34	0.38			0.49	0.70			0.40			0.26	
Control Delay		50.0	9.2			51.6	12.3			40.0			8.6	
Queue Delay		0.0	0.0			0.0	0.0			0.0			0.0	
Total Delay		50.0	9.2			51.6	12.3			40.0			8.6	
LOS	D	A			D	B				D			A	
Approach Delay		10.7					13.5			40.0			8.6	
Approach LOS		B					B			D			A	
Queue Length 50th (ft)		27	114			48	330			17			0	
Queue Length 95th (ft)		65	208			107	648			34			19	
Internal Link Dist (ft)		440					551			282			152	
Turn Bay Length (ft)		200				350								
Base Capacity (vph)		363	3370			381	3739			223			298	
Starvation Cap Reductn		0	0			0	0			0			0	
Spillback Cap Reductn		0	0			0	0			0			0	
Storage Cap Reductn		0	0			0	0			0			0	
Reduced v/c Ratio		0.13	0.38			0.22	0.70			0.20			0.15	

Intersection Summary

Area Type: Other

Cycle Length: 117

Actuated Cycle Length: 93.8

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 12.9

Intersection LOS: B

Intersection Capacity Utilization 73.2%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: Marine Boulevard/Shepard Street & Lynnway (Route 1A)



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑↑↑↑			↑↑↑↑↑				↑↑↑↑↑			↑↑↑↑↑
Volume (veh/h)	0	1005	145	0	2550	145	0	0	75	0	0	40
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.83	0.83	0.83	0.95	0.95	0.95	0.75	0.75	0.75	0.68	0.68	0.68
Hourly flow rate (vph)	0	1211	175	0	2684	153	0	0	100	0	0	59
Pedestrians		3			1			1			10	
Lane Width (ft)		12.0			12.0			12.0			12.0	
Walking Speed (ft/s)		4.0			4.0			4.0			4.0	
Percent Blockage		0			0			0			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)		631			594							
pX, platoon unblocked	0.44			0.95			0.46	0.46	0.95	0.46	0.46	0.44
vC, conflicting volume	2847			1387			2256	4146	392	3174	4157	984
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	745			1168			0	2928	126	829	2952	0
tC, single (s)	4.2			4.1			7.7	6.7	7.1	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.3			2.2			3.6	4.1	3.4	3.6	4.1	3.4
p0 queue free %	100			100			100	100	88	100	100	87
cM capacity (veh/h)	361			567			401	6	838	102	6	465
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	NB 1	SB 1			
Volume Total	346	346	346	348	1074	1074	689	100	59			
Volume Left	0	0	0	0	0	0	0	0	0			
Volume Right	0	0	0	175	0	0	153	100	59			
cSH	1700	1700	1700	1700	1700	1700	1700	838	465			
Volume to Capacity	0.20	0.20	0.20	0.20	0.63	0.63	0.41	0.12	0.13			
Queue Length 95th (ft)	0	0	0	0	0	0	0	10	11			
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	9.9	13.9			
Lane LOS								A	B			
Approach Delay (s)	0.0				0.0			9.9	13.9			
Approach LOS								A	B			
Intersection Summary												
Average Delay		0.4										
Intersection Capacity Utilization		63.5%			ICU Level of Service			B				
Analysis Period (min)		15										

11942.03 :: Blossom Street Improvements
3: Kingman Street/Jughandle & Lynnway (Route 1A)

2014 Existing Conditions with Ferry Traffic
Timing Plan: Weekday Morning

Lane Group	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations														
Volume (vph)	15	15	985	35	5	60	2775	165	30	5	35	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0			275		0	0		50	0			
Storage Lanes	1		0		1		0	0		1	0			
Taper Length (ft)	25				25				25		25			
Satd. Flow (prot)	0	1687	4818	0	0	1787	5084	0	0	1199	1062	0	0	0
Flt Permitted		0.950				0.950				0.959				
Satd. Flow (perm)	0	1687	4818	0	0	1783	5084	0	0	1198	1062	0	0	0
Right Turn on Red				Yes				Yes			Yes			Yes
Satd. Flow (RTOR)					6			11			67			
Link Speed (mph)						30		30		30		30		
Link Distance (ft)							410			266		157		
Travel Time (s)							9.3			6.0		3.6		
Conf. Peds. (#/hr)			9		6		6		9	1		4	4	1
Peak Hour Factor	0.84	0.84	0.84	0.84	0.94	0.94	0.94	0.94	0.72	0.72	0.72	0.92	0.92	0.92
Heavy Vehicles (%)	7%	7%	7%	7%	1%	1%	1%	1%	52%	52%	52%	2%	2%	2%
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	36	1215	0	0	69	3128	0	0	49	49	0	0	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Split	NA	Prot			
Protected Phases	1	1	6		5	5	2		4	4	4			
Permitted Phases														
Detector Phase	1	1	6		5	5	2		4	4	4			
Switch Phase														
Minimum Initial (s)	7.0	7.0	7.0		7.0	7.0	7.0		7.0	7.0	7.0			
Minimum Split (s)	11.0	11.0	26.0		11.0	11.0	26.0		26.0	26.0	26.0			
Total Split (s)	24.0	24.0	24.0		24.0	24.0	24.0		34.0	34.0	34.0			
Total Split (%)	29.3%	29.3%	29.3%		29.3%	29.3%	29.3%		41.5%	41.5%	41.5%			
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.0	3.0			
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0	0.0			0.0	0.0				
Total Lost Time (s)	4.0	4.0			4.0	4.0			4.0	4.0				
Lead/Lag	Lead	Lead	Lag		Lead	Lead	Lag							
Lead-Lag Optimize?														
Recall Mode	None	None	Max		None	None	Min		None	None	None			
Act Effct Green (s)		7.3	31.7			7.6	34.1			9.5	9.5			
Actuated g/C Ratio		0.15	0.65			0.16	0.70			0.20	0.20			
v/c Ratio		0.14	0.39			0.25	0.88			0.21	0.19			
Control Delay		23.3	10.0			23.5	20.2			19.1	5.4			
Queue Delay		0.0	0.0			0.0	0.0			0.0	0.0			
Total Delay		23.3	10.0			23.5	20.2			19.1	5.4			
LOS	C	B			C	C			B	A				
Approach Delay			10.4				20.2			12.3				
Approach LOS			B				C			B				
Queue Length 50th (ft)		9	82			17	186			12	0			
Queue Length 95th (ft)		35	182			58	#797			27	8			
Internal Link Dist (ft)			514				330			186		77		
Turn Bay Length (ft)						275					50			
Base Capacity (vph)		712	3134			754	3567			759	697			
Starvation Cap Reductn		0	0			0	0			0	0			
Spillback Cap Reductn		0	0			0	0			0	0			
Storage Cap Reductn		0	0			0	0			0	0			
Reduced v/c Ratio		0.05	0.39			0.09	0.88			0.06	0.07			

Intersection Summary

Area Type: Other
 Cycle Length: 82
 Actuated Cycle Length: 48.7
 Natural Cycle: 100
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.88
 Intersection Signal Delay: 17.4 Intersection LOS: B
 Intersection Capacity Utilization 73.8% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 3: Kingman Street/Jughandle & Lynnway (Route 1A)



	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations														
Volume (vph)	10	100	2305	15	55	0	1180	50	10	5	10	45	0	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		0		350		0	0		0	0	0	0	0
Storage Lanes	1		0		1		0	0		0	0	0	0	0
Taper Length (ft)	25				25				25				25	
Satd. Flow (prot)	0	1770	5080	0	0	1752	5001	0	0	1694	0	0	1729	0
Flt Permitted		0.950				0.950				0.878			0.832	
Satd. Flow (perm)	0	1768	5080	0	0	1752	5001	0	0	1515	0	0	1492	0
Right Turn on Red				Yes				Yes			Yes			Yes
Satd. Flow (RTOR)			1				8			25			65	
Link Speed (mph)			30				30			30			30	
Link Distance (ft)			520				631			362			232	
Travel Time (s)			11.8				14.3			8.2			5.3	
Conf. Peds. (#/hr)		2		2		2		2	3				3	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.92	0.92	0.92	0.92	0.38	0.38	0.38	0.82	0.82	0.82
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	112	2367	0	0	60	1337	0	0	65	0	0	73	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	1	1	6		5	5	2			4			4	
Permitted Phases													4	
Detector Phase	1	1	6		5	5	2		4	4		4	4	
Switch Phase														
Minimum Initial (s)	7.0	7.0	15.0		7.0	7.0	15.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	12.0	12.0	20.0		12.0	12.0	20.0		27.0	27.0		27.0	27.0	
Total Split (s)	25.0	25.0	65.0		25.0	25.0	65.0		27.0	27.0		27.0	27.0	
Total Split (%)	21.4%	21.4%	55.6%		21.4%	21.4%	55.6%		23.1%	23.1%		23.1%	23.1%	
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		4.0	4.0		4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0			0.0	0.0			0.0	0.0		0.0		
Total Lost Time (s)		5.0	5.0			5.0	5.0			6.0			6.0	
Lead/Lag	Lead	Lead	Lag		Lead	Lead	Lag							
Lead-Lag Optimize?														
Recall Mode	None	None	Max		None	None	Max		None	None		None	None	
Act Effct Green (s)		10.6	67.2			8.4	60.9			9.7				9.7
Actuated g/C Ratio		0.11	0.71			0.09	0.65			0.10			0.10	
v/c Ratio		0.56	0.65			0.38	0.41			0.37			0.34	
Control Delay		53.0	12.2			51.1	10.2			33.6			17.2	
Queue Delay		0.0	0.0			0.0	0.0			0.0			0.0	
Total Delay		53.0	12.2			51.1	10.2			33.6			17.2	
LOS	D	B			D	B			C			B		
Approach Delay			14.0				12.0			33.6			17.2	
Approach LOS			B				B			C			B	
Queue Length 50th (ft)		64	277			34	125			23			4	
Queue Length 95th (ft)		134	551			85	257			17			37	
Internal Link Dist (ft)			440				551			282			152	
Turn Bay Length (ft)		200			350									
Base Capacity (vph)		380	3623			377	3232			361			387	
Starvation Cap Reductn		0	0			0	0			0			0	
Spillback Cap Reductn		0	0			0	0			0			0	
Storage Cap Reductn		0	0			0	0			0			0	
Reduced v/c Ratio		0.29	0.65			0.16	0.41			0.18			0.19	
Intersection Summary														
Area Type:			Other											
Cycle Length:	117													
Actuated Cycle Length:		94.3												
Natural Cycle:	80													
Control Type:		Actuated-Uncoordinated												
Maximum v/c Ratio:	0.65													
Intersection Signal Delay: 13.7					Intersection LOS: B									
Intersection Capacity Utilization 71.2%						ICU Level of Service C								
Analysis Period (min) 15														

Splits and Phases: 1: Marine Boulevard/Shepard Street & Lynnway (Route 1A)



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑↑↑↑↑↑↑			↑↑↑↑↑↑↑↑				↑↑↑↑↑↑↑↑			↑↑↑↑↑↑↑↑
Volume (veh/h)	0	2360	55	0	1360	165	0	0	115	0	0	40
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.99	0.99	0.99	0.91	0.91	0.91	0.83	0.83	0.83	0.66	0.66	0.66
Hourly flow rate (vph)	0	2384	56	0	1495	181	0	0	139	0	0	61
Pedestrians		1			1							9
Lane Width (ft)		12.0			12.0							12.0
Walking Speed (ft/s)		4.0			4.0							4.0
Percent Blockage		0			0							1
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)		631			594							
pX, platoon unblocked	0.81			0.78			0.87	0.87	0.78	0.87	0.87	0.81
vC, conflicting volume	1685			2439			2971	4096	625	2330	4034	599
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1014			1412			874	2163	0	138	2091	0
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	100	84	100	100	93
cM capacity (veh/h)	550			372			197	40	841	582	43	861
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	NB 1	SB 1			
Volume Total	681	681	681	396	598	598	480	139	61			
Volume Left	0	0	0	0	0	0	0	0	0			
Volume Right	0	0	0	56	0	0	181	139	61			
cSH	1700	1700	1700	1700	1700	1700	1700	841	861			
Volume to Capacity	0.40	0.40	0.40	0.23	0.35	0.35	0.28	0.16	0.07			
Queue Length 95th (ft)	0	0	0	0	0	0	0	15	6			
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.1	9.5			
Lane LOS								B	A			
Approach Delay (s)	0.0				0.0			10.1	9.5			
Approach LOS								B	A			
Intersection Summary												
Average Delay		0.5										
Intersection Capacity Utilization		49.2%			ICU Level of Service			A				
Analysis Period (min)		15										

11942.03 :: Blossom Street Improvements
3: Kingman Street/Jughandle & Lynnway (Route 1A)

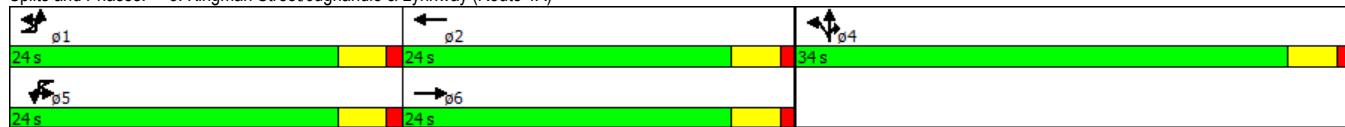
2014 Existing Conditions with Ferry Traffic
Timing Plan: Weekday Evening

Lane Group	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations														
Volume (vph)	55	10	2370	20	15	55	1285	35	140	0	100	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0			275		0	0		50	0		0	
Storage Lanes	1		0		1		0	0		1	0		0	
Taper Length (ft)		25				25			25			25		
Satd. Flow (prot)	0	1787	5130	0	0	1752	5013	0	0	1736	1553	0	0	0
Flt Permitted		0.950				0.950			0.950					
Satd. Flow (perm)	0	1787	5130	0	0	1752	5013	0	0	1734	1553	0	0	0
Right Turn on Red				Yes				Yes		Yes			Yes	
Satd. Flow (RTOR)		1				5					148			
Link Speed (mph)		30				30			30			30		
Link Distance (ft)		594				410			266			157		
Travel Time (s)		13.5				9.3			6.0			3.6		
Confl. Peds. (#/hr)		1		1		1		1	1		5	5		1
Peak Hour Factor	0.96	0.96	0.96	0.96	0.94	0.94	0.94	0.94	0.57	0.57	0.57	0.92	0.92	0.92
Heavy Vehicles (%)	1%	1%	1%	1%	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	67	2490	0	0	75	1404	0	0	246	175	0	0	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Split	NA	Prot			
Protected Phases	1	1	6		5	5	2		4	4	4			
Permitted Phases														
Detector Phase	1	1	6		5	5	2		4	4	4			
Switch Phase														
Minimum Initial (s)	7.0	7.0	7.0		7.0	7.0	7.0		7.0	7.0	7.0			
Minimum Split (s)	11.0	11.0	26.0		11.0	11.0	26.0		26.0	26.0	26.0			
Total Split (s)	24.0	24.0	24.0		24.0	24.0	24.0		34.0	34.0	34.0			
Total Split (%)	29.3%	29.3%	29.3%		29.3%	29.3%	29.3%		41.5%	41.5%	41.5%			
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.0	3.0			
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0	0.0			0.0	0.0				
Total Lost Time (s)		4.0	4.0			4.0	4.0			4.0	4.0			
Lead/Lag	Lead	Lead	Lag		Lead	Lead	Lag							
Lead-Lag Optimize?														
Recall Mode	None	None	Max		None	None	Min		None	None	None			
Act Effct Green (s)		7.7	23.3			7.9	23.5			12.5	12.5			
Actuated g/C Ratio	0.15	0.46			0.16	0.47			0.25	0.25				
v/c Ratio	0.25	1.05			0.27	0.60			0.57	0.35				
Control Delay	25.1	54.3			25.2	14.1			23.2	7.2				
Queue Delay	0.0	0.0			0.0	0.0			0.0	0.0				
Total Delay	25.1	54.3			25.2	14.1			23.2	7.2				
LOS	C	D			C	B			C	A				
Approach Delay		53.5				14.7			16.6					
Approach LOS		D				B			B					
Queue Length 50th (ft)	19	~355			22	122			70	7				
Queue Length 95th (ft)	58	#619			62	238			77	12				
Internal Link Dist (ft)		514				330			186		77			
Turn Bay Length (ft)					275					50				
Base Capacity (vph)	738	2369			723	2331			1075	1018				
Starvation Cap Reductn	0	0			0	0			0	0				
Spillback Cap Reductn	0	0			0	0			0	0				
Storage Cap Reductn	0	0			0	0			0	0				
Reduced v/c Ratio	0.09	1.05			0.10	0.60			0.23	0.17				

Intersection Summary

Area Type: Other
 Cycle Length: 82
 Actuated Cycle Length: 50.5
 Natural Cycle: 80
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 1.05
 Intersection Signal Delay: 37.1
 Intersection LOS: D
 Intersection Capacity Utilization 77.1%
 ICU Level of Service D
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 3: Kingman Street/Jughandle & Lynnway (Route 1A)



11942.03 :: Blossom Street Improvements

1: Marine Boulevard/Shepard Street & Lynnway (Route 1A)

Option 1

Timing Plan: Weekday Morning

Lane Group	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations														
Volume (vph)	5	35	1040	15	25	5	2395	50	10	10	10	25	0	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			0		350		0	0		0	0		0
Storage Lanes	1			0		1		0	0		0	0		0
Taper Length (ft)	25					25			25					25
Satd. Flow (prot)	0	1687	4836	0	0	1770	5067	0	0	1056	0	0	1328	0
Flt Permitted		0.950				0.950				0.871			0.802	
Satd. Flow (perm)	0	1686	4836	0	0	1767	5067	0	0	934	0	0	1094	0
Right Turn on Red				Yes				Yes			Yes			Yes
Satd. Flow (RTOR)		3					4			15			65	
Link Speed (mph)		30					30			30			30	
Link Distance (ft)		520					631			362			232	
Travel Time (s)		11.8					14.3			8.2			5.3	
Conf. Peds. (#/hr)		8		3		3		8	2				2	
Peak Hour Factor	0.83	0.83	0.83	0.83	0.94	0.94	0.94	0.94	0.65	0.65	0.65	1.00	1.00	1.00
Heavy Vehicles (%)	7%	7%	7%	7%	2%	2%	2%	2%	69%	69%	69%	30%	30%	30%
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	48	1271	0	0	32	2601	0	0	45	0	0	45	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	1	1	6		5	5	2			4			4	
Permitted Phases								4			4			
Detector Phase	1	1	6		5	5	2		4	4		4		4
Switch Phase														
Minimum Initial (s)	7.0	7.0	15.0		7.0	7.0	15.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	12.0	12.0	20.0		12.0	12.0	20.0		27.0	27.0		27.0	27.0	
Total Split (s)	25.0	25.0	65.0		25.0	25.0	65.0		27.0	27.0		27.0	27.0	
Total Split (%)	21.4%	21.4%	55.6%		21.4%	21.4%	55.6%		23.1%	23.1%		23.1%	23.1%	
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		4.0	4.0		4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0			0.0	0.0			0.0	0.0		0.0		
Total Lost Time (s)		5.0	5.0			5.0	5.0			6.0			6.0	
Lead/Lag	Lead	Lead	Lag		Lead	Lead	Lag							
Lead-Lag Optimize?														
Recall Mode	None	None	Max		None	None	Max		None	None		None	None	
Act Effct Green (s)		7.9	65.8			7.4	65.3			9.9			9.9	
Actuated g/C Ratio	0.09	0.73				0.08	0.73			0.11			0.11	
v/c Ratio	0.32	0.36				0.22	0.71			0.39			0.25	
Control Delay	47.8	7.3			46.4	12.7			38.1			8.4		
Queue Delay	0.0	0.0			0.0	0.0			0.0			0.0		
Total Delay	47.8	7.3			46.4	12.7			38.1			8.4		
LOS	D	A			D	B			D			A		
Approach Delay		8.8				13.1			38.1			8.4		
Approach LOS		A				B			D			A		
Queue Length 50th (ft)	27	105			18	328			17			0		
Queue Length 95th (ft)	64	186			52	648			33			19		
Internal Link Dist (ft)		440				551			282			152		
Turn Bay Length (ft)	200				350									
Base Capacity (vph)	380	3545			399	3684			232			309		
Starvation Cap Reductn	0	0			0	0			0			0		
Spillback Cap Reductn	0	0			0	0			0			0		
Storage Cap Reductn	0	0			0	0			0			0		
Reduced v/c Ratio	0.13	0.36			0.08	0.71			0.19			0.15		

Intersection Summary

Area Type: Other

Cycle Length: 117

Actuated Cycle Length: 89.8

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.71

Intersection Signal Delay: 11.9

Intersection LOS: B

Intersection Capacity Utilization 63.2%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Marine Boulevard/Shepard Street & Lynnway (Route 1A)



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	0	1005	95	50	2500	145	0	0	75	0	0	40
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.83	0.83	0.83	0.92	0.95	0.95	0.75	0.75	0.75	0.68	0.68	0.68
Hourly flow rate (vph)	0	1211	114	54	2632	153	0	0	100	0	0	59
Pedestrians		3			1			1			10	
Lane Width (ft)		12.0			12.0			12.0			12.0	
Walking Speed (ft/s)		4.0			4.0			4.0			4.0	
Percent Blockage		0			0			0			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)		631			594							
pX, platoon unblocked	0.44			0.91			0.49	0.49	0.91	0.49	0.49	0.44
vC, conflicting volume	2794			1326			2317	4172	463	3331	4153	967
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	661			1012			0	2587	63	869	2548	0
tC, single (s)	4.2			4.1			7.7	6.7	7.1	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.3			2.2			3.6	4.1	3.4	3.6	4.1	3.4
p0 queue free %	100			91			100	100	89	100	100	87
cM capacity (veh/h)	392			619			396	10	878	95	11	470
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1			
Volume Total	484	484	357	54	1053	1053	679	100	59			
Volume Left	0	0	0	54	0	0	0	0	0			
Volume Right	0	0	114	0	0	0	153	100	59			
cSH	1700	1700	1700	619	1700	1700	1700	878	470			
Volume to Capacity	0.28	0.28	0.21	0.09	0.62	0.62	0.40	0.11	0.13			
Queue Length 95th (ft)	0	0	0	7	0	0	0	10	11			
Control Delay (s)	0.0	0.0	0.0	11.4	0.0	0.0	0.0	9.6	13.8			
Lane LOS				B				A	B			
Approach Delay (s)	0.0			0.2				9.6	13.8			
Approach LOS								A	B			
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utilization		62.5%		ICU Level of Service				B				
Analysis Period (min)		15										

11942.03 :: Blossom Street Improvements
 3: Kingman Street/Jughandle & Lynnway (Route 1A)

Option 1
 Timing Plan: Weekday Morning

	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Configurations														
Volume (vph)	15	15	985	35	5	60	2775	165	30	5	35	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250		0		275		0	0		50	0		0	
Storage Lanes	1		0		1		0	0		1	0		0	
Taper Length (ft)	25				25				25			25		
Satd. Flow (prot)	0	1687	4818	0	0	1787	5084	0	0	1199	1062	0	0	0
Flt Permitted		0.950				0.950				0.959				
Satd. Flow (perm)	0	1687	4818	0	0	1783	5084	0	0	1198	1062	0	0	0
Right Turn on Red				Yes				Yes			Yes			Yes
Satd. Flow (RTOR)					6			11				67		
Link Speed (mph)						30		30			30		30	
Link Distance (ft)							410			266		157		
Travel Time (s)					13.5			9.3			6.0		3.6	
Conf. Peds. (#/hr)		9		6		6		9	1		4	4		1
Peak Hour Factor	0.84	0.84	0.84	0.84	0.94	0.94	0.94	0.94	0.72	0.72	0.72	0.92	0.92	0.92
Heavy Vehicles (%)	7%	7%	7%	7%	1%	1%	1%	1%	52%	52%	52%	2%	2%	2%
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	36	1215	0	0	69	3128	0	0	49	49	0	0	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Split	NA	Prot			
Protected Phases	1	1	6		5	5	2		4	4	4			
Permitted Phases														
Detector Phase	1	1	6		5	5	2		4	4	4			
Switch Phase														
Minimum Initial (s)	7.0	7.0	7.0		7.0	7.0	7.0		7.0	7.0	7.0			
Minimum Split (s)	11.0	11.0	26.0		11.0	11.0	26.0		26.0	26.0	26.0			
Total Split (s)	24.0	24.0	24.0		24.0	24.0	24.0		34.0	34.0	34.0			
Total Split (%)	29.3%	29.3%	29.3%		29.3%	29.3%	29.3%		41.5%	41.5%	41.5%			
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.0	3.0			
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0	0.0			0.0	0.0				
Total Lost Time (s)		4.0	4.0			4.0	4.0			4.0	4.0			
Lead/Lag	Lead	Lead	Lag		Lead	Lead	Lag							
Lead-Lag Optimize?														
Recall Mode	None	None	Max		None	None	Min		None	None	None			
Act Effct Green (s)		7.3	31.7			7.6	34.1			9.5	9.5			
Actuated g/C Ratio		0.15	0.65			0.16	0.70			0.20	0.20			
v/c Ratio		0.14	0.39			0.25	0.88			0.21	0.19			
Control Delay		23.3	10.0			23.5	20.2			19.1	5.4			
Queue Delay		0.0	0.0			0.0	0.0			0.0	0.0			
Total Delay		23.3	10.0			23.5	20.2			19.1	5.4			
LOS	C	B			C	C			B	A				
Approach Delay		10.4				20.2				12.3				
Approach LOS		B				C				B				
Queue Length 50th (ft)		9	82			17	186			12	0			
Queue Length 95th (ft)		35	182			58	#797			27	8			
Internal Link Dist (ft)		514				330			186		77			
Turn Bay Length (ft)		250			275					50				
Base Capacity (vph)		712	3134			754	3567			759	697			
Starvation Cap Reductn		0	0			0	0			0	0			
Spillback Cap Reductn		0	0			0	0			0	0			
Storage Cap Reductn		0	0			0	0			0	0			
Reduced v/c Ratio		0.05	0.39			0.09	0.88			0.06	0.07			

Intersection Summary

Area Type:	Other
Cycle Length:	82
Actuated Cycle Length:	48.7
Natural Cycle:	100
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.88
Intersection Signal Delay:	17.4
Intersection LOS:	B
Intersection Capacity Utilization:	73.8%
ICU Level of Service:	D
Analysis Period (min):	15
#	95th percentile volume exceeds capacity, queue may be longer.
	Queue shown is maximum after two cycles.

Splits and Phases: 3: Kingman Street/Jughandle & Lynnway (Route 1A)



11942.03 :: Blossom Street Improvements

1: Marine Boulevard/Shepard Street & Lynnway (Route 1A)

Option 1

Timing Plan: Weekday Evening

Lane Group	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations														
Volume (vph)	10	100	2305	15	40	0	1180	50	10	5	10	45	0	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900		1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		0		350		0	0		0	0	0	0	0
Storage Lanes	1		0		1		0	0		0	0	0	0	0
Taper Length (ft)	25				25				25			25		
Satd. Flow (prot)	0	1770	5080	0	0	1752	5001	0	0	1694	0	0	1729	0
Flt Permitted		0.950				0.950				0.878			0.831	
Satd. Flow (perm)	0	1768	5080	0	0	1752	5001	0	0	1515	0	0	1491	0
Right Turn on Red				Yes				Yes			Yes			Yes
Satd. Flow (RTOR)			1				8			25			65	
Link Speed (mph)		30				30			30			30		
Link Distance (ft)		520				631			362			232		
Travel Time (s)		11.8					14.3			8.2			5.3	
Conf. Peds. (#/hr)		2		2		2		2	3				3	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.92	0.92	0.92	0.92	0.38	0.38	0.38	0.82	0.82	0.82
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	112	2367	0	0	43	1337	0	0	65	0	0	73	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	1	1	6		5	5	2			4			4	
Permitted Phases									4			4		
Detector Phase	1	1	6		5	5	2		4	4		4		4
Switch Phase														
Minimum Initial (s)	7.0	7.0	15.0		7.0	7.0	15.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	12.0	12.0	20.0		12.0	12.0	20.0		27.0	27.0		27.0	27.0	
Total Split (s)	25.0	25.0	65.0		25.0	25.0	65.0		27.0	27.0		27.0	27.0	
Total Split (%)	21.4%	21.4%	55.6%		21.4%	21.4%	55.6%		23.1%	23.1%		23.1%	23.1%	
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		4.0	4.0		4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0			0.0	0.0			0.0	0.0		0.0		
Total Lost Time (s)		5.0	5.0			5.0	5.0			6.0			6.0	
Lead/Lag	Lead	Lead	Lag		Lead	Lead	Lag							
Lead-Lag Optimize?														
Recall Mode	None	None	Max		None	None	Max		None	None		None	None	
Act Effct Green (s)		10.6	70.3				7.8	60.9		9.7				9.7
Actuated g/C Ratio		0.11	0.75				0.08	0.65		0.10				0.10
v/c Ratio		0.56	0.62				0.30	0.41		0.37				0.34
Control Delay		53.0	10.4			49.9	10.2			33.6				17.2
Queue Delay		0.0	0.0			0.0	0.0			0.0				0.0
Total Delay		53.0	10.4			49.9	10.2			33.6				17.2
LOS	D	B			D	B			C					B
Approach Delay			12.3				11.5			33.6				17.2
Approach LOS			B				B			C				B
Queue Length 50th (ft)		64	270			25	125			23				4
Queue Length 95th (ft)		134	531			67	257			17				37
Internal Link Dist (ft)			440				551			282				152
Turn Bay Length (ft)		200			350									
Base Capacity (vph)		380	3789			377	3232			361				387
Starvation Cap Reductn		0	0			0	0			0				0
Spillback Cap Reductn		0	0			0	0			0				0
Storage Cap Reductn		0	0			0	0			0				0
Reduced v/c Ratio		0.29	0.62			0.11	0.41			0.18				0.19

Intersection Summary

Area Type: Other

Cycle Length: 117

Actuated Cycle Length: 94.3

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.62

Intersection LOS: B

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: Marine Boulevard/Shepard Street & Lynnway (Route 1A)



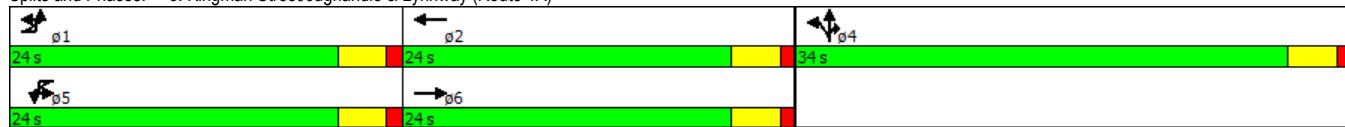
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	0	2360	40	15	1345	165	0	0	115	0	0	40
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.99	0.99	0.99	0.92	0.91	0.91	0.83	0.83	0.83	0.66	0.66	0.66
Hourly flow rate (vph)	0	2384	40	16	1478	181	0	0	139	0	0	61
Pedestrians		1			1							9
Lane Width (ft)		12.0			12.0							12.0
Walking Speed (ft/s)		4.0			4.0							4.0
Percent Blockage		0			0							1
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)		631			594							
pX, platoon unblocked	0.81			0.73			0.82	0.82	0.73	0.82	0.82	0.81
vC, conflicting volume	1668			2424			2991	4105	816	2544	4035	593
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1004			1648			1108	2462	0	565	2376	0
tC, single (s)	4.1			4.1			7.5	6.5	6.9	7.6	6.6	7.0
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			94			100	100	82	100	100	93
cM capacity (veh/h)	556			283			120	23	789	257	25	863
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1			
Volume Total	954	954	517	16	591	591	477	139	61			
Volume Left	0	0	0	16	0	0	0	0	0			
Volume Right	0	0	40	0	0	0	181	139	61			
cSH	1700	1700	1700	283	1700	1700	1700	789	863			
Volume to Capacity	0.56	0.56	0.30	0.06	0.35	0.35	0.28	0.18	0.07			
Queue Length 95th (ft)	0	0	0	5	0	0	0	16	6			
Control Delay (s)	0.0	0.0	0.0	18.5	0.0	0.0	0.0	10.5	9.5			
Lane LOS				C				B	A			
Approach Delay (s)	0.0			0.2				10.5	9.5			
Approach LOS								B	A			
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization		60.6%		ICU Level of Service				B				
Analysis Period (min)		15										

Lane Group	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations														
Volume (vph)	55	10	2370	20	15	55	1285	35	140	0	100	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250		0		275		0	0		50	0		0	
Storage Lanes	1		0		1		0	0		1	0		0	
Taper Length (ft)	25				25				25		25			
Satd. Flow (prot)	0	1787	5130	0	0	1752	5013	0	0	1736	1553	0	0	0
Flt Permitted		0.950				0.950				0.950				
Satd. Flow (perm)	0	1787	5130	0	0	1752	5013	0	0	1734	1553	0	0	0
Right Turn on Red				Yes				Yes			Yes			Yes
Satd. Flow (RTOR)			1				5				148			
Link Speed (mph)			30				30			30		30		
Link Distance (ft)			594				410			266		157		
Travel Time (s)			13.5				9.3			6.0		3.6		
Conf. Peds. (#/hr)		1		1		1		1	1	1	5	5	1	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.94	0.94	0.94	0.94	0.57	0.57	0.57	0.92	0.92	0.92
Heavy Vehicles (%)	1%	1%	1%	1%	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	67	2490	0	0	75	1404	0	0	246	175	0	0	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Split	NA	Prot			
Protected Phases	1	1	6		5	5	2		4	4	4			
Permitted Phases														
Detector Phase	1	1	6		5	5	2		4	4	4			
Switch Phase														
Minimum Initial (s)	7.0	7.0	7.0		7.0	7.0	7.0		7.0	7.0	7.0			
Minimum Split (s)	11.0	11.0	26.0		11.0	11.0	26.0		26.0	26.0	26.0			
Total Split (s)	24.0	24.0	24.0		24.0	24.0	24.0		34.0	34.0	34.0			
Total Split (%)	29.3%	29.3%	29.3%		29.3%	29.3%	29.3%		41.5%	41.5%	41.5%			
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.0	3.0			
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0	0.0			0.0	0.0				
Total Lost Time (s)	4.0	4.0			4.0	4.0			4.0	4.0				
Lead/Lag	Lead	Lead	Lag		Lead	Lead	Lag							
Lead-Lag Optimize?														
Recall Mode	None	None	Max		None	None	Min		None	None	None			
Act Effct Green (s)		7.7	23.3			7.9	23.5			12.5	12.5			
Actuated g/C Ratio	0.15	0.46			0.16	0.47			0.25	0.25				
v/c Ratio	0.25	1.05			0.27	0.60			0.57	0.35				
Control Delay	25.1	54.3			25.2	14.1			23.2	7.2				
Queue Delay	0.0	0.0			0.0	0.0			0.0	0.0				
Total Delay	25.1	54.3			25.2	14.1			23.2	7.2				
LOS	C	D			C	B			C	A				
Approach Delay		53.5				14.7			16.6					
Approach LOS		D				B			B					
Queue Length 50th (ft)	19	~355			22	122			70	7				
Queue Length 95th (ft)	58	#619			62	238			77	12				
Internal Link Dist (ft)		514				330			186		77			
Turn Bay Length (ft)		250			275					50				
Base Capacity (vph)	738	2369			723	2331			1075	1018				
Starvation Cap Reductn	0	0			0	0			0	0				
Spillback Cap Reductn	0	0			0	0			0	0				
Storage Cap Reductn	0	0			0	0			0	0				
Reduced v/c Ratio	0.09	1.05			0.10	0.60			0.23	0.17				

Intersection Summary

Area Type:	Other
Cycle Length:	82
Actuated Cycle Length:	50.5
Natural Cycle:	80
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	1.05
Intersection Signal Delay: 37.1	Intersection LOS: D
Intersection Capacity Utilization 77.1%	ICU Level of Service D
Analysis Period (min) 15	
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 3: Kingman Street/Jughandle & Lynnway (Route 1A)



Preliminary Cost Estimates



Vanasse Hangen Brustlin, Inc.

**Transportation
Land Development
Environmental
Services**

101 Walnut Street
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617 924 1770

**LYNN - LYNNWAY AT BLOSSOM ST
CONCEPTUAL CONSTRUCTION COST ESTIMATE
SUMMARY**

Construction Items

<u>Description</u>	<u>Unit Price</u>	<u>Quantity</u>	<u>Total Cost</u>
Full Depth Pavement	\$86.00 /SY	210 SY	\$18,060.00
Full Depth Pavement - Less than 4.0'	\$117.00 /SY	10 SY	\$1,170.00
Cement Concrete Median	\$73.00 /SY	510 SY	\$37,230.00
Granite Curb	\$39.00 /FT	920 FT	\$35,880.00
Signing & Striping	\$2,800.00 /LS	1 LS	\$2,800.00
Drainage	\$11,500.00 /LS	1 LS	\$11,500.00
	SUBTOTAL:		\$106,640.00
	Police Detail (10%)		\$10,664
	Mobilization (3%)		\$3,199
	Construction Traffic Management (5%)		\$5,332
	TOTAL:		\$125,835
	Contingencies (15%):		\$18,875
	Construction TOTAL:		\$144,710

NOTE:

1. Prices were determined from the MassDOT Weighted Average Bid Prices web site.



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**LYNN - LYNNWAY AT BLOSSOM ST
CONCEPTUAL CONSTRUCTION COST ESTIMATE
SUMMARY - FLASHING WARNING BEACON ALTERNATIVE**

Construction Items

<u>Description</u>	<u>Unit Price</u>	<u>Quantity</u>	<u>Total Cost</u>
Full Depth Pavement	\$86.00 /SY	210 SY	\$18,060.00
Full Depth Pavement - Less than 4.0'	\$117.00 /SY	10 SY	\$1,170.00
Cement Concrete Median	\$73.00 /SY	510 SY	\$37,230.00
Granite Curb	\$39.00 /FT	920 FT	\$35,880.00
Signing & Striping	\$2,800.00 /LS	1 LS	\$2,800.00
Drainage	\$11,500.00 /LS	1 LS	\$11,500.00
Flashing Warning Beacon	\$44,000.00 /LS	1 LS	\$44,000.00
	SUBTOTAL:		\$150,640.00
	Police Detail (10%)		\$15,064
	Mobilization (3%)		\$4,519
	Construction Traffic Management (5%)		\$7,532
	TOTAL:		\$177,755
	Contingencies (15%):		\$26,663
	Construction TOTAL:		\$204,418

NOTE:

1. Prices were determined from the MassDOT Weighted Average Bid Prices web site.



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**LYNN - LYNNWAY AT BLOSSOM ST
CONCEPTUAL CONSTRUCTION COST ESTIMATE
SUMMARY - TRAFFIC SIGNAL ALTERNATIVE**

Construction Items

<u>Description</u>	<u>Unit Price</u>	<u>Quantity</u>	<u>Total Cost</u>
Full Depth Pavement	\$86.00 /SY	210 SY	\$18,060.00
Full Depth Pavement - Less than 4.0'	\$117.00 /SY	10 SY	\$1,170.00
Cement Concrete Median	\$73.00 /SY	510 SY	\$37,230.00
Granite Curb	\$39.00 /FT	920 FT	\$35,880.00
Signing & Striping	\$2,800.00 /LS	1 LS	\$2,800.00
Drainage	\$11,500.00 /LS	1 LS	\$11,500.00
Traffic Signal	\$86,500.00 /LS	1 LS	\$86,500.00
	SUBTOTAL:		\$193,140.00
	Police Detail (10%)		\$19,314
	Mobilization (3%)		\$5,794
	Construction Traffic Management (5%)		\$9,657
	TOTAL:		\$227,905
	Contingencies (15%):		\$34,186
	Construction TOTAL:		\$262,091

NOTE:

1. Prices were determined from the MassDOT Weighted Average Bid Prices web site.

Signal Warrant Worksheet

2009 MUTCD

TRAFFIC SIGNAL WARRANT ANALYSIS (VOLUME BASED)

Intersection: [Lynnway \(Route 1A\) at Blossom Street](#)

Major Street Direction: [Eastbound-Westbound](#) ▾

Year: 2014 **Condition:** Existing with Ferry Traffic (WBL assumed to be minor street approach)

Operating speed on major roadway: 35 mph **Required approach volumes**

Number of approaches: 4

Warrant 1	EIGHT-HOUR VEHICULAR VOLUME	Major Street : 3 Lane(s) on each approach	Adjusted Minimum*	
			Minimum*	Adjusted Minimum**
Warrant 1A	MINIMUM VEHICULAR VOLUME (8 hours of day)	Minor Street : 1 Lane(s) on each approach	600	600
			150	150
Warrant 1B	INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day)	Major Street : 3 Lane(s) on each approach	900	900
		Minor Street : 1 Lane(s) on each approach	75	75
80 PERCENT SATISFACTION OF WARRANT 1A AND WARRANT 1B			Warrant 1A	Warrant 1B
	Major Street : 3 Lane(s) on each approach		480	720
	Minor Street : 1 Lane(s) on each approach		120	60

Warrant 2 FOUR HOUR VEHICULAR VOLUME

Major Street : 3 Lane(s) on each approach
Minor Street : 1 Lane(s) on each approach

If "verify" indicated, see Figure 4C-1 or 4C-2.
25 = accuracy of regression equations

Warrant 3 PEAK HOUR VOLUME

Major Street : 3 Lane(s) on each approach
Minor Street : 1 Lane(s) on each approach

If "verify" indicated, see Figure 4C-3 or 4C-4.
25 = accuracy of regression equations

Hour	Entering Vol. Minor Road+	Entering Vol. on Major Road		Tot. Ent. Vol. On Major Rd	Meets the following volume-based warrants?				
		Eastbound	Westbound		1A	1B	80%(1A&1B)	2	3
6:00 - 7:00 AM	0	0	0	0	No	No	No	0	0
7:00 - 8:00 AM	50	1100	0	1100	No	No	No	0	0
8:00 - 9:00 AM	0	0	0	0	No	No	No	0	0
9:00 - 10:00 AM	0	0	0	0	No	No	No	0	0
10:00 - 11:00 AM	0	0	0	0	No	No	No	0	0
11:00 - 12:00 AM	0	0	0	0	No	No	No	0	0
12:00 - 1:00 PM	0	0	0	0	No	No	No	0	0
1:00 - 2:00 PM	0	0	0	0	No	No	No	0	0
2:00 - 3:00 PM	0	0	0	0	No	No	No	0	0
3:00 - 4:00 PM	0	0	0	0	No	No	No	0	0
4:00 - 5:00 PM	0	0	0	0	No	No	No	0	0
5:00 - 6:00 PM	15	2400	0	2400	No	No	No	0	0
6:00 - 7:00 PM	0	0	0	0	No	No	No	0	0
					No	No	No	No	No
					Warrants Met?	1	2	3	
						NO	No	No	No

*From the criteria described for the warrant in the MUTCD.

**If the operating speed is higher than 40mph then the volumes can be adjusted to 70%. (If no adjusted minimum, the minimum from the previous column is shown)

+If more than one approach, report the approach that has the higher volume.

NON-VOLUME-BASED WARRANTS

Warrant 4, Minimum Pedestrian Volume:

No

Peak Four Hour Pedestrian Volumes:
(non-concurrent)

0
0
0
0

Warrant 5, School Crossing:

See MUTCD for details.

Warrant 7, Crash Experience: No

of accidents "correctable by signalization" occurring in the last 12 months:

0

Warrant 6, Coordinated Signal System:

See MUTCD for details.

Warrant 8, Roadway Network:

See MUTCD for details.



Route 1A/Lynnway/Carroll Parkway

**Federal Fiscal Year 2015 Priority Corridors for
Long-Range Transportation Plan (LRTP)
Needs Assessment**

April 17, 2015

Seth Asante

Boston Region Metropolitan Planning Organization

Purpose of Priority Corridors Study

- Improve safety
- Reduce congestion
- Provide efficient traffic operations
- Encourage multimodal accommodation
- Provide better access management
- Promote healthy transportation

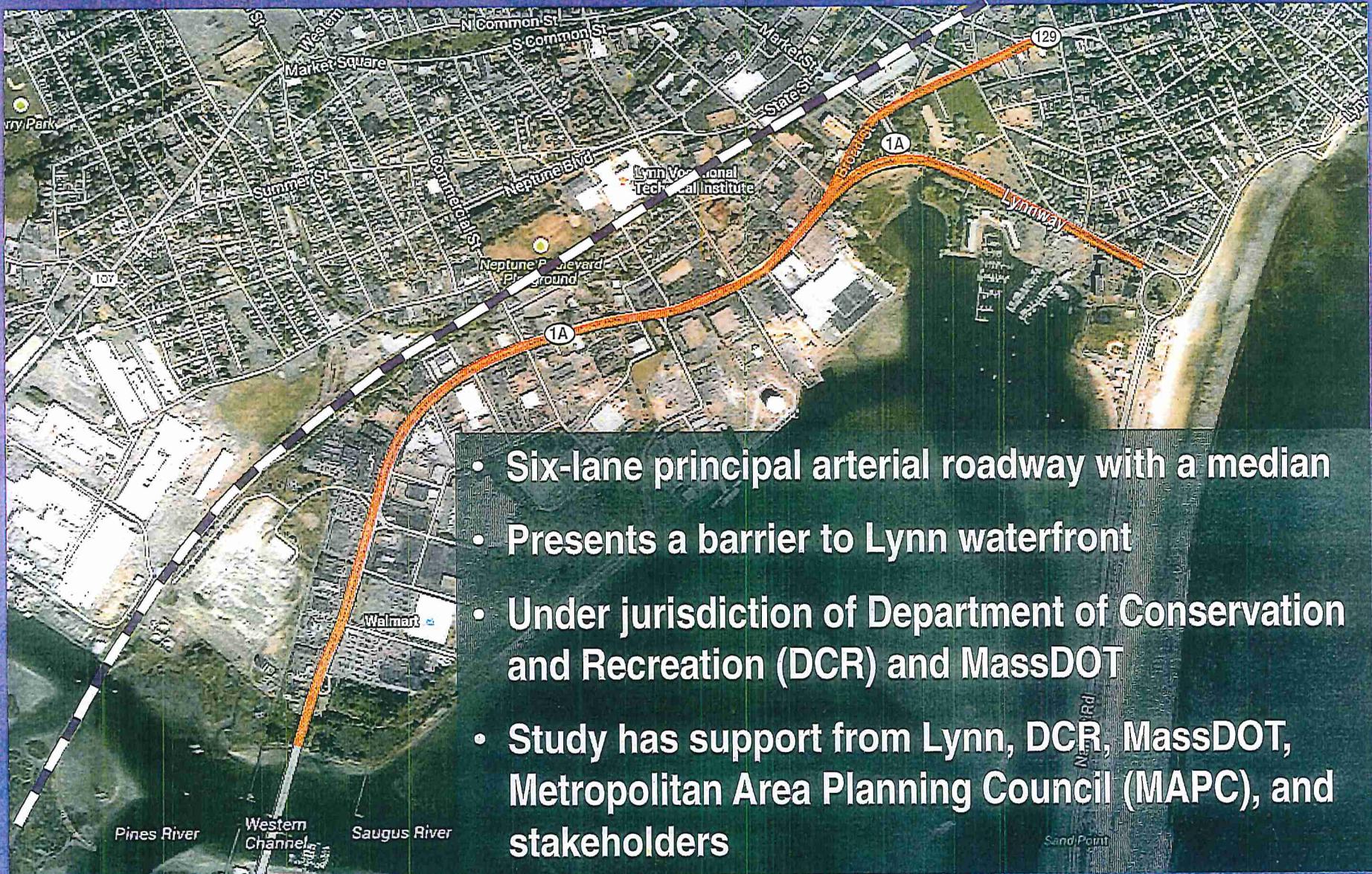


Selection Criteria

- **Safety conditions**
- **Multimodal significance**
- **Congestion**
- **Regional or subregional significance**
- **Implementation potential**
- **Regional equity**



Route 1A/Lynnway/Carroll Parkway



Route 1A/Lynnway/Carroll Parkway and Lynn Waterfront

- **Vision**
- **Goals and objectives**

Vision for Lynn Waterfront

- **Connect the City with the waterfront**
 - **Create open spaces along the waterfront**
 - **Design mixed use neighborhood**
 - **Transform the Lynnway into a pedestrian friendly boulevard**
 - **Upgrade the traffic system to be more pedestrian-friendly**
 - **Create a walkable, livable community that promotes human interaction**
-



Goals and Objectives

Route 1A/Lynnway/Carroll Parkway

- Serve through or regional traffic
- Balance the needs of through traffic with the needs:
 - Pedestrians
 - Bicyclists
 - Local traffic access to the waterfront



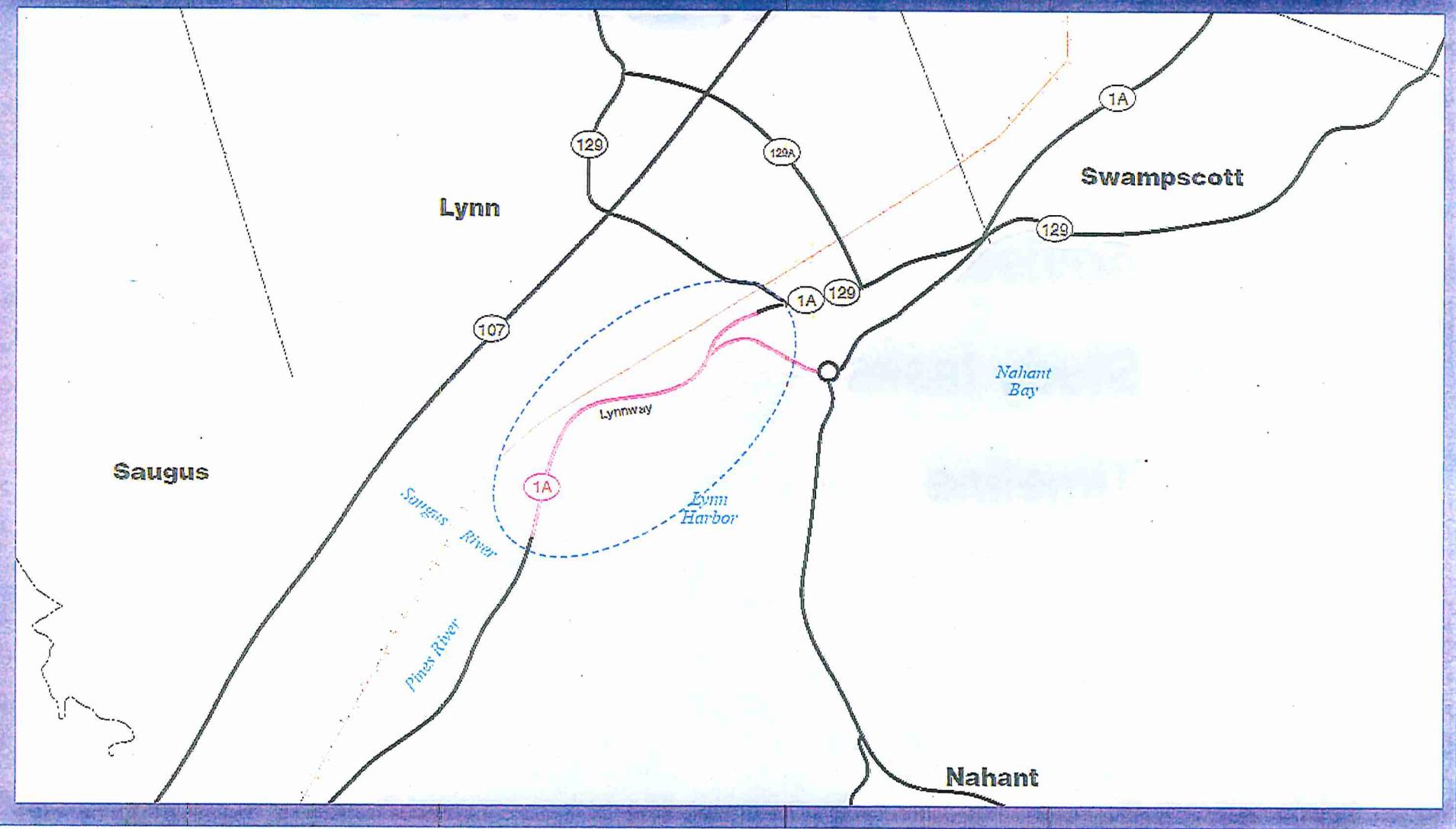
Transportation and Land Use Interaction



Scope of Study

- **Study area**
- **Advisory task force**
- **Study tasks**
- **Timeline**

Study Area Map



Study Limits



Advisory Task Force

- **City of Lynn**
- **Department of Conservation and Recreation**
- **MassDOT Highway Division District 4**
- **MassDOT Office of Planning and Transportation**
- **Metropolitan Area Planning Council**



Study Tasks

- **Data collection**
- **Existing conditions analyses**
- **Forecast future traffic**
- **Develop and analyze alternatives**
- **Document study**



Data Collection

- Land use
- Traffic volumes
- Speed data
- Crash data
- Traffic signal data
- Bus service data



Existing Conditions

- **Land Uses**
 - Inventory of corridor uses and businesses in the project area
 - Identification of major development opportunities, including vacant or underused sites.
- **Pedestrian and bicycle needs**
 - Sidewalks, crosswalks, and pedestrian signals
 - Bike lanes and detectors and signs



Existing Conditions

- Safety conditions
 - Total crashes (vehicular, pedestrian, and bicyclist)
 - Crash rates (intersection and roadway)
 - Collision diagrams
- Traffic patterns and operations
 - Peak period conditions
 - Levels of service (delays and queues)
 - Travel speeds
 - Signal equipment



Future Traffic Forecasts

- **Forecast horizon (2030 or 2040)**
- **Regional travel demand model set**
- **Calibrated for 101 cities and towns
in the Boston Region MPO area**
- **Adopted for the Long-Range
Transportation Plan**



Develop and Analyze Alternatives

- **As many as 3 alternatives**
- **Road diet concepts (reconfiguring roadway to improve safety and operations)**
- **Complete street concepts (safe, convenient, and comfortable access for all users)**
- **Traffic circulation (efficient traffic operations)**



Document Study

- **Present products of study tasks**
- **Prepare draft document for review**
- **Address comments and feedback**
- **Finalize report**



Estimated Schedule



Thank you!

Questions?



To: City of Lynn

Date: September 9, 2015

Memorandum

Project #: 11942.03

From: Nicolette Hastings, PE
Kathleen Keen, EIT

Re: Lynnway at Blossom Street
Conceptual Improvements

Introduction

VHB evaluated a conceptual improvement option for the intersection of Lynnway (Route 1A) at Blossom Street in Lynn, Massachusetts. The evaluation considered impacts to traffic flow and operations along the Lynnway as a result of a potential improvement option at Blossom Street to improve access to the Lynn Commuter Ferry Terminal. This memorandum summarizes the results of this evaluation and includes a sketch level conceptual improvement plan and order of magnitude cost estimates for the City's consideration.

Existing Conditions

The existing conditions evaluation consisted of field inventories (including signal timing/phasing), the collection of peak period traffic volumes, and an operational analysis at the study area intersections.

Study Area

This memorandum evaluates the Lynnway in the vicinity of Blossom Street. The Lynnway is a principal arterial under the Department of Conservation and Recreation (DCR) jurisdiction and is oriented in an east-west direction within the study area. The Lynnway consists of three travel lanes in each direction with no shoulders. Eastbound and westbound traffic are separated by a concrete median. The posted speed limit in the vicinity of Blossom Street is 35 miles per hour (mph) eastbound and westbound. The study area consists of the following three intersections:

- **Lynnway (Route 1A) at Shepard Street/Marine Boulevard:** Signalized intersection with Shepard Street intersecting the Lynnway from the north and Marine Boulevard intersecting the Lynnway from the south. The Lynnway eastbound and westbound approaches consist of an exclusive left-turn storage lane, two through travel lanes, and a shared through/right-turn lane. There is a u-turn restriction for trucks on the Lynnway eastbound approach. The Shepard Street southbound approach and the Marine Boulevard northbound approach both consist of a single general purpose lane accommodating all movements. Sidewalks exist on both sides of the Lynnway and Shepard Street. Crosswalks are present across the eastbound, northbound, and southbound approaches.
- **Lynnway (Route 1A) at Blossom Street:** Unsignalized intersection with Blossom Street intersecting the Lynnway from the north and south. The Blossom Street northbound and southbound approaches are under STOP control and are right-in/right-out with the Lynnway eastbound and westbound traffic separated by a concrete median. The Lynnway eastbound approach consists of an exclusive left-turn storage lane for the downstream intersection with Kingman Street, two through travel lanes, and a shared through/right-turn lane. The Lynnway westbound approach consists of two through travel lanes and a shared through/right-turn lane. Sidewalks are present on all corners of the intersection with the exception of the western side of Blossom Street south of the Lynnway. Crosswalks exist on both the northbound and

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southbound approaches of Blossom Street; there are no pedestrian accommodations across the Lynnway at this location

- **Lynnway (Route 1A) at Kingman Street/jughandle:** Signalized intersection with Kingman Street intersecting the Lynnway from the south and a one-way northbound driveway intersecting the Lynnway from the north. The Lynnway eastbound and westbound approaches consist of an exclusive left-turn storage lane, two through travel lanes, and a shared through/right-turn lane. The Kingman Street approach consists of shared through/left-turn lane and an exclusive right-turn lane. Sidewalks are present on both sides of the Lynnway and crosswalks exist across the westbound, northbound, and southbound approaches.

Traffic Volumes

Manual turning movement counts (TMCs) were conducted at the three study area intersections during the weekday morning from 7:00 AM to 9:00 AM and during the weekday evening from 4:00 PM to 6:00 PM in November 2014. It should be noted that the TMCs were conducted during a time when the Lynn ferry service was not operational. A review of the data collected indicate the weekday morning peak hour is from 7:00 AM to 8:00 AM and the weekday evening peak hour is from 4:45 PM to 5:45 PM.

Lynn Commuter Ferry Service

The Lynn commuter ferry terminal is located at the southern end of Blossom Street. The Lynn to Boston ferry service operated seasonally in 2014 (the first year of a two year pilot program), from mid-May to mid-September. During this period, service was offered on weekday mornings and weekday evenings with two trips departing from Lynn in the morning and two trips departing from Boston during the evening.

The City of Lynn provided ferry ridership data for the month of September 2014 which was used to establish typical weekday morning and weekday evening peak hour traffic volumes at the study area intersections while the ferry service is operational. Table 1 shows the total number of vehicle trips added to the weekday morning and weekday evening existing traffic volumes.

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Table 1 Peak Hour Ferry Ridership

	Weekday Morning	Weekday Evening
Enter	56	13
<u>Exit</u>	4	<u>64</u>
Total	60	77

Source: Lynn ferry ridership data, September 2014.

The trips in Table 1 were distributed onto the existing roadway network assuming 90 percent of the trips are coming to/from points north/east and 10 percent of trips are coming to/from points south/west. These assumptions are based on information provided by the City of Lynn. Figure 1 shows the resulting 2014 Existing Conditions (with ferry traffic) traffic volumes during the weekday morning and weekday evening peak hours at the study area intersections.

Traffic Operations Analysis

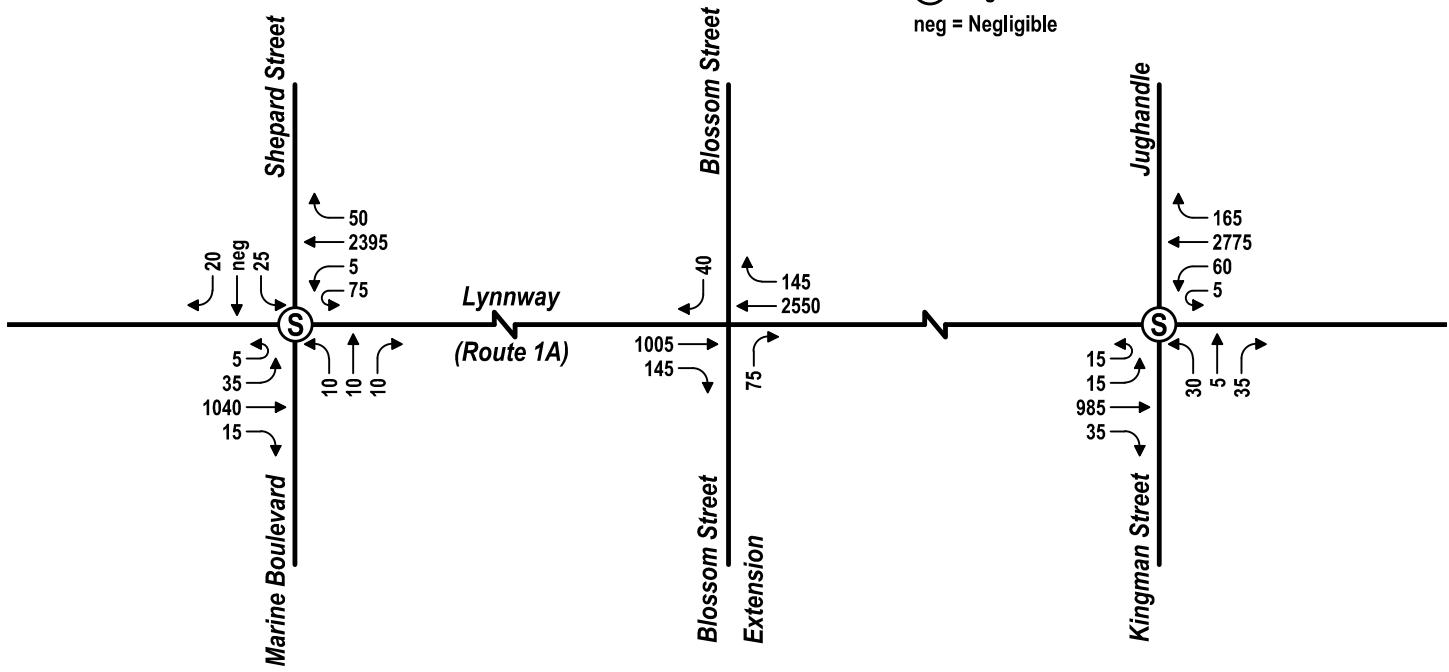
VHB conducted capacity analyses using SYNCHRO 8 software. The percentile delay method (SYNCHRO outputs) was used to evaluate how the signalized intersections accommodate the traffic demands, consistent with current MassDOT standards. Methods from the 2000 Highway Capacity Manual (HCM)¹ were used to evaluate how the unsignalized intersections accommodate the traffic demands as the HCM 2010 methodology does not support more than three through lanes on a major street approach. The capacity analyses were used to evaluate operations of the study area intersections and to provide a baseline to assess the operational benefits of the improvement options. Tables 2 and 3 show the results of the operations analysis at the signalized and unsignalized study area intersections, respectively.

¹ 2000 Highway Capacity Manual; Transportation Research Board: Washington, D.C.

WEEKDAY MORNING PEAK HOUR

(S) Signalized Intersection

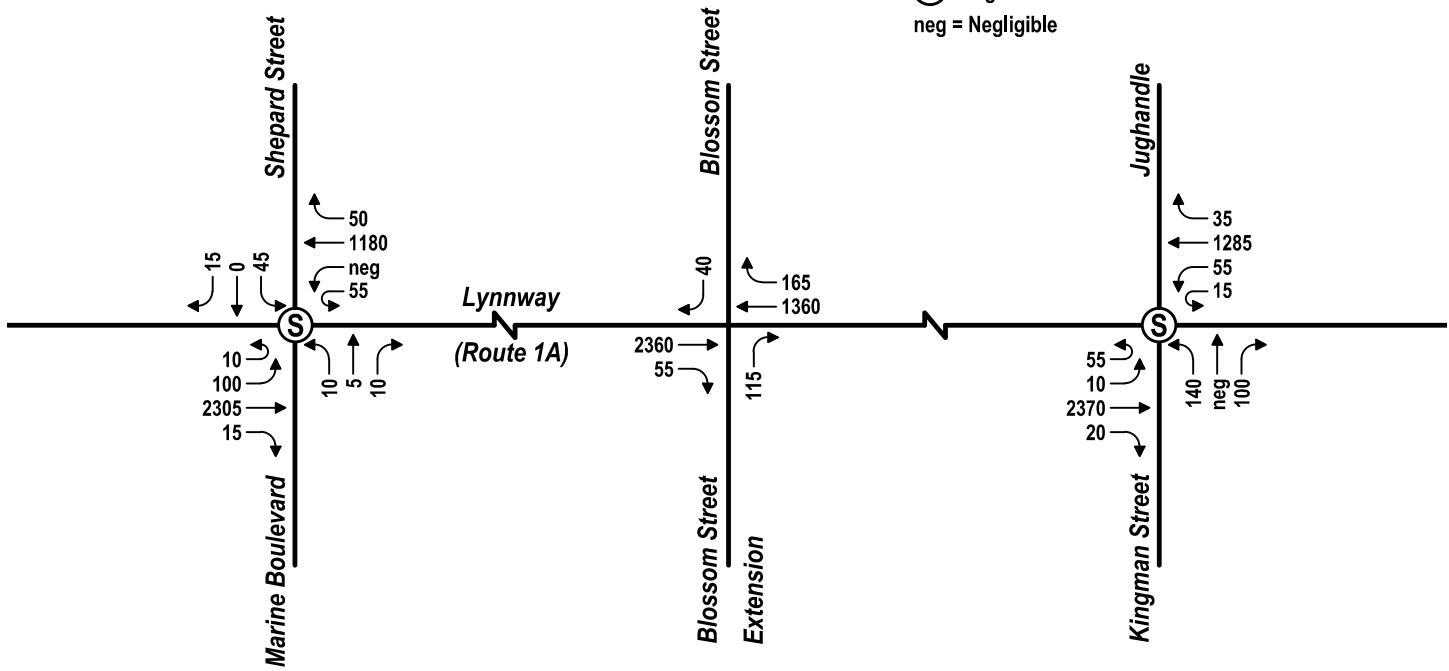
neg = Negligible



WEEKDAY EVENING PEAK HOUR

(S) Signalized Intersection

neg = Negligible



Vanasse Hangen Brustlin, Inc.

2014 Existing Conditions
Peak Hour Traffic Volumes
with Ferry Traffic

Figure 1



Not to Scale

Blossom Street Improvements
Lynn, Massachusetts

Table 2 Signalized Intersection Capacity Analyses

2014 Existing Conditions with Ferry Traffic											
Intersection	Movement	Weekday Morning Peak Hour					Weekday Evening Peak Hour				
		V/C ¹	Del ²	LOS ³	50 Q ⁴	95 Q ⁵	V/C	Del	LOS	50 Q	95 Q
1: Lynnway (Route 1A) at Shepard Street/ Marine Boulevard	Lynnway EB L	0.34	50	D	27	65	0.56	53	D	64	134
	Lynnway EB T/R	0.38	9	A	114	208	0.65	12	B	277	551
	Lynnway WB L	0.49	52	D	48	107	0.38	51	D	34	85
	Lynnway WB T/R	0.70	12	B	330	648	0.41	10	B	125	257
	Marine Blvd NB L/T/R	0.40	40	D	17	34	0.37	34	C	23	17
	Shepard St SB L/T/R	0.26	9	A	0	19	0.34	17	B	4	37
	Overall	13	B				14	B			
3: Lynnway (Route 1A) at Kingman Street/ jughandle	Lynnway EB L	0.14	23	C	9	35	0.25	25	C	19	58
	Lynnway EB T/R	0.39	10	B	82	182	1.05	54	D	~355	#619
	Lynnway WB L	0.25	24	C	17	58	0.27	25	C	22	62
	Lynnway WB T/R	0.88	20	C	186	#797	0.60	14	B	122	238
	Kingman St NB L/T	0.21	19	B	12	27	0.57	23	C	70	77
	Kingman St NB R	0.19	5	A	0	8	0.35	7	A	7	12
	Overall	17	B				37	D			

Source: VHB, Inc. using SYNCHRO 8 software

1 Volume-to-capacity ratio, based on theoretical capacity.

2 Average delay, in seconds per vehicle

3 Level of service

4 50th percentile queue length estimate, in feet

5 95th percentile queue length estimate, in feet.

~ Volume exceeds capacity, queue is theoretically infinite

95th percentile volumes exceeds capacity, queue may be longer

Table 3 Unsignalized Intersection Capacity Analyses

2014 Existing Conditions with Ferry Traffic											
Intersection	Critical Movements	Weekday Morning Peak Hour					Weekday Evening Peak Hour				
		D ¹	V/C ²	Del ³	LOS ⁴	95 Q ⁵	D	V/C	Del	LOS	95 Q
2: Lynnway (Route 1A) at Blossom Street	Blossom St Ext NB R	75	0.12	10	A	10	115	0.16	10	B	15
	Blossom St SB R	40	0.13	14	B	11	40	0.07	10	A	6

Source: VHB, Inc. using SYNCHRO 8 software.

Note: The HCM 2010 methodology does not support more than three through lanes on a major street approach, results reported are based on HCM 2000 methodology.

1 Demand, in vehicles

2 Volume-to-capacity ratio, based on theoretical capacity.

3 Average delay, in seconds per vehicle

4 Level of service

5 95th percentile queue length estimate, in feet



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As shown in Tables 2 and 3, all three study area intersections operate at an acceptable LOS D or better during both the weekday morning and weekday evening peak hours.

Conceptual Improvement Evaluation

In coordination with the City of Lynn, VHB developed and evaluated a conceptual improvement option to add a Lynnway westbound left-turn lane at Blossom Street. The following section includes a discussion of the conceptual improvement option, summarizes impacts to traffic operations, and provides a preliminary lump sum cost estimate.

As shown in Figure 2, the conceptual improvement option includes a westbound left-turn lane along the Lynnway at the Blossom Street intersection. To accommodate the left-turn lane, the existing eastbound left-turn lane at Kingman Street would be shortened and the existing median would be relocated. The westbound left-turn lane at Blossom Street would be approximately 100 feet and the remaining eastbound left-turn lane at Kingman Street would be approximately 310 feet. The revised median design shown in the concept plan would serve to discourage left-turn and through movements from Blossom Street in both directions. In addition, a diverter island is proposed on the Blossom Street southbound approach to reinforce right-in/right-out operations. The preliminary lump sum cost estimate for this improvement option is approximately \$145,000.

As part of the conceptual improvement option, commuter ferry traffic currently making a westbound u-turn at the Lynnway and Shepard Street/Maine Boulevard intersection would be relocated to make a westbound left-turn at Blossom Street. Figure 3 shows the resulting weekday morning and weekday evening peak hour traffic volumes.

Tables 4 and 5 summarize the traffic operations under the conceptual improvement option for the signalized and unsignalized intersections, respectively. This analysis assumes that the Lynnway at Blossom Street intersection would remain unsignalized; no signal timing changes at the Lynnway and Shepard Street/Marine Boulevard intersection are assumed. No changes in operations would occur at intersection of Lynnway and Kingman Street/jughandle. As shown in Table 4, overall operations at the intersection of Lynnway and Shepard Street/Marine Boulevard are expected to improve during both peak hours under the conceptual improvement option, which may be attributed to the reduction in the westbound u-turn volume. As shown in Table 5, the westbound left-turn lane at the intersection of Lynnway and Blossom Street is expected to operate at LOS C or better during both peak hours with minimal queues under the conceptual improvement option. It should be noted that the revised turn lane lengths at the intersections with Blossom Street and Kingman Street discussed above can adequately accommodate the projected weekday morning and weekday evening peak hour queue demands at both locations.

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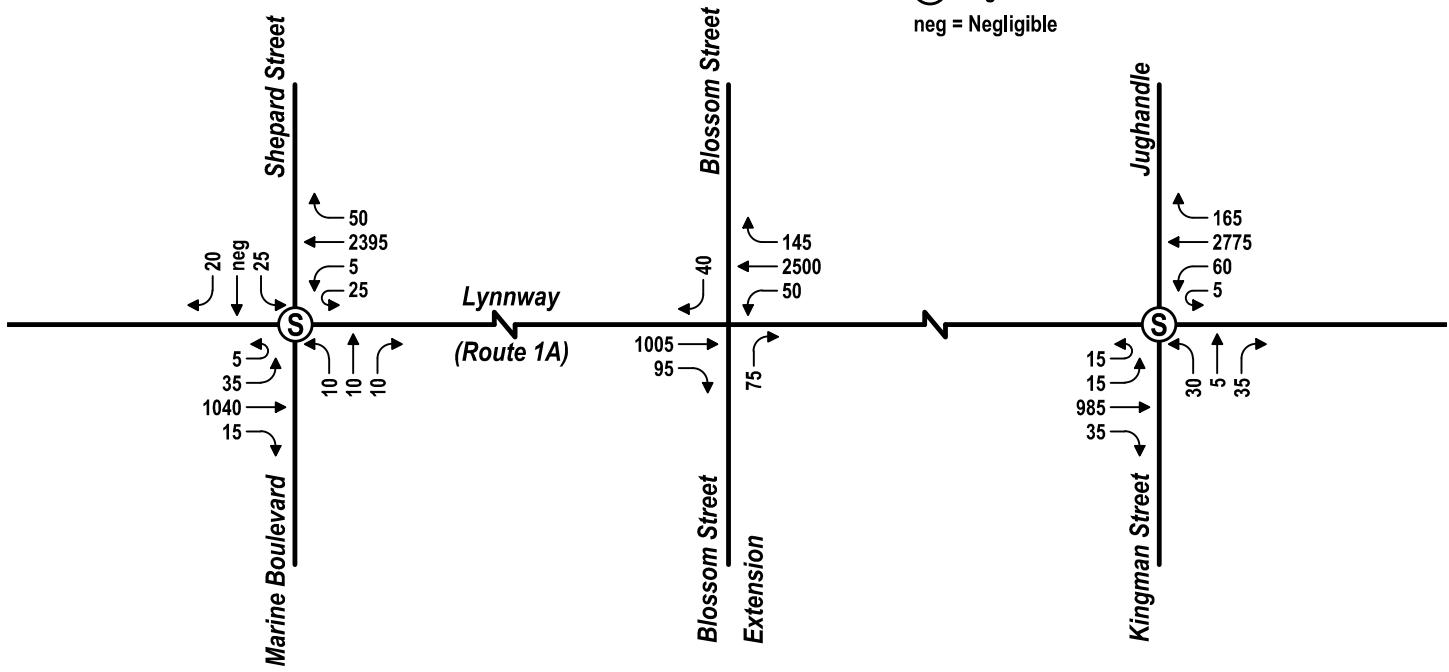


Figure 2
Conceptual Improvement Option
Blossom Street Improvements
Lynn, Massachusetts

WEEKDAY MORNING PEAK HOUR

(S) Signalized Intersection

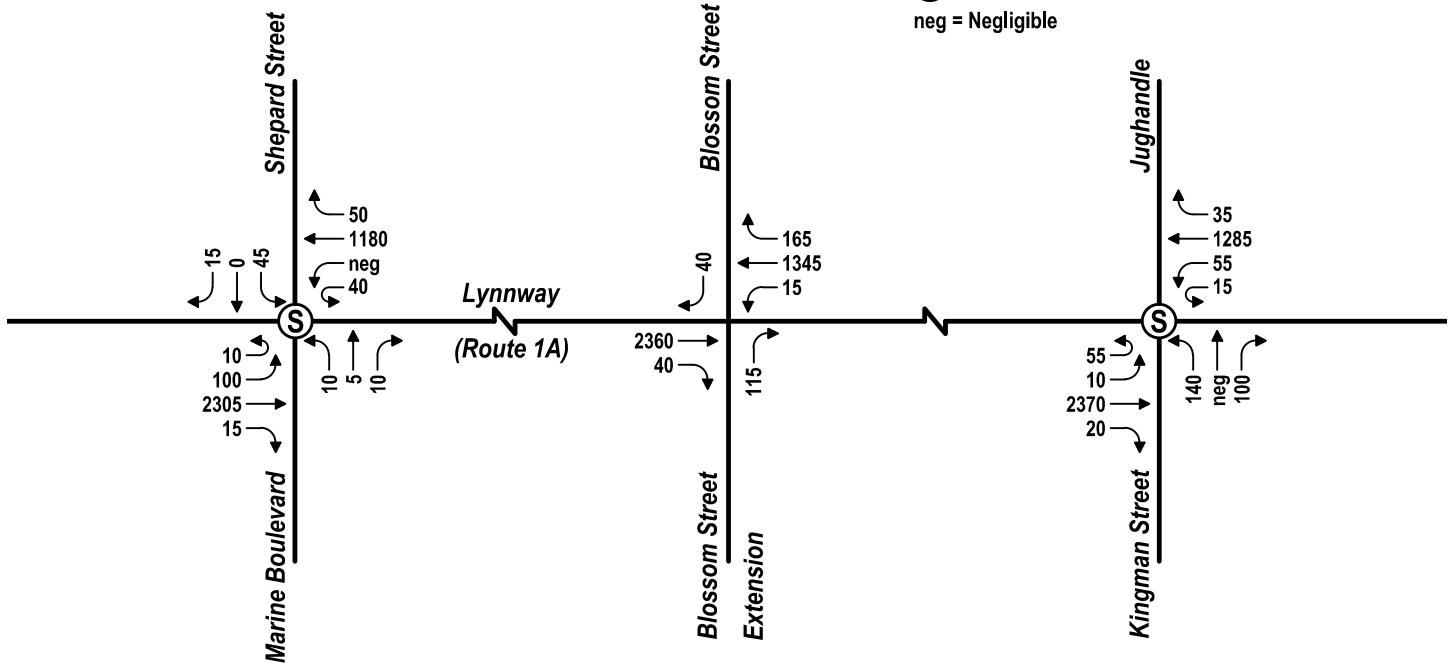
neg = Negligible



WEEKDAY EVENING PEAK HOUR

(S) Signalized Intersection

neg = Negligible



Vanasse Hangen Brustlin, Inc.

2014 Existing Conditions
Peak Hour Traffic Volumes
with Ferry Traffic and Future Geometry

Figure 3



Not to Scale

Blossom Street Improvements
Lynn, Massachusetts



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Table 4 Signalized Intersection Capacity Analyses

Intersection	Movement	Improvement Option: Westbound Left-Turn Lane									
		Weekday Morning Peak Hour					Weekday Evening Peak Hour				
		V/C ¹	Del ²	LOS ³	50 Q ⁴	95 Q ⁵	V/C	Del	LOS	50 Q	95 Q
1: Lynnway (Route 1A) at Shepard Street/ Marine Boulevard	Lynnway EB L	0.32	48	D	27	64	0.56	53	D	64	134
	Lynnway EB T/R	0.36	7	A	105	186	0.62	10	B	270	531
	Lynnway WB L	0.22	46	D	18	52	0.30	50	D	25	67
	Lynnway WB T/R	0.71	13	B	328	648	0.41	10	B	125	257
	Marine Blvd NB L/T/R	0.39	38	D	17	33	0.37	34	C	23	17
	Shepard St SB L/T/R	0.25	8	A	0	19	0.34	17	B	4	37
Overall		12	B					13	B		

Source: VHB, Inc. using SYNCHRO 8 software

1 Volume-to-capacity ratio, based on theoretical capacity.

2 Average delay, in seconds per vehicle

3 Level of service

4 50th percentile queue length estimate, in feet

5 95th percentile queue length estimate, in feet.

~ Volume exceeds capacity, queue is theoretically infinite

95th percentile volumes exceeds capacity, queue may be longer

Table 5 Unsignalized Intersection Capacity Analyses

Intersection	Critical Movements	Improvement Option: Westbound Left-Turn Lane									
		Morning Peak Hour					Evening Peak Hour				
D ¹	V/C ²	Del ³	LOS ⁴	95 Q ⁵	D	V/C	Del	LOS	95 Q		
2: Lynnway (Route 1A) at Blossom Street	Lynnway WB L	50	0.09	11	B	7	15	0.06	19	C	5
	Blossom St Ext NB R	75	0.11	10	A	10	115	0.18	11	B	16
	Blossom St SB R	40	0.13	14	B	11	40	0.07	10	A	6

Source: VHB, Inc. using SYNCHRO 8 software.

Note: The HCM 2010 methodology does not support more than three through lanes on a major street approach, results reported are based on HCM 2000 methodology.

1 Demand, in vehicles

2 Volume-to-capacity ratio, based on theoretical capacity.

3 Average delay, in seconds per vehicle

4 Level of service

5 95th percentile queue length estimate, in feet

To increase awareness of the Lynnway westbound left-turn movement at Blossom Street, an intersection control beacon could be installed in accordance with the standards established in the Manual on Uniform Traffic Control Devices² (MUTCD). The beacon would have flashing yellow signal heads directed toward the Lynnway eastbound and have flashing red signal heads directed towards the Blossom Street northbound approach and Lynnway westbound

² MUTCD, Part 4 – Highway Traffic Signals, USDOT/FHWA, December 2009.



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left-turn lane. The preliminary lump sum cost estimate to install an intersection control beacon is approximately \$60,000, in addition to the cost of the geometric improvements.

Signal Warrant Analysis

VHB performed a traffic signal warrant analysis at the intersection of Lynnway at Blossom Street, assuming the improvements proposed as part of the conceptual improvement option. The MUTCD lists specific criteria, or warrants, for the consideration of installation of a traffic signal at an intersection. The MUTCD also notes that, "the satisfaction of a traffic signal warrant or warrants shall not, in itself, require the installation of a traffic control signal." The traffic signal warrant analysis provides guidance as to locations where signals would not be appropriate and locations where they could be considered further.

A traffic signal warrant analysis was performed for the volume-based Warrant 3: Peak Hour Volume for the weekday morning and weekday evening peak hours. The warrant analysis was performed in a manner that considers the Lynnway westbound left-turn volume as the "minor street" volume and the opposing traffic on the Lynnway eastbound as the "major street" volume, as allowed by the MUTCD. The warrant is not met at the intersection for either of the peak hours. The City of Lynn has noted the possibility of expanding ferry services in the future. It is recommended that traffic volumes at the intersection continue to be monitored and if the signal warrant is met in the future, the installation of a signal at the Lynnway and Blossom Street intersection should be re-evaluated. The preliminary lump sum cost estimate for the installation of a signal is approximately \$117,000.

Summary

VHB, in coordination with the City of Lynn, has developed and evaluated a conceptual improvement option along the Lynnway within the vicinity of Blossom Street which would improve access to the Lynn Commuter Ferry Terminal. The improvement option considers a Lynnway westbound left-turn lane at Blossom Street.

VHB evaluated traffic operations under the conceptual improvement option. Overall traffic operations are expected to improve within the study area and minimal queues are expected in the proposed Lynnway westbound left-turn lane at Blossom Street. In addition, VHB performed a signal warrant analysis for the intersection of Lynnway at Blossom Street assuming the improvement proposed as part of the conceptual improvement option; a signal is not warranted at this time. The intersection should continue to be monitored and if the warrant is met in the future with potential increased ferry service, the installation of a traffic signal at this location should be re-evaluated. While a traffic signal is not currently warranted, an intersection control beacon could be installed to increase awareness of the intersection.

The preliminary lump sum cost estimate of the geometric improvements under the conceptual improvement option is approximately \$145,000. The installation of an intersection control beacon would have an additional cost of approximately \$60,000. In the future, if a traffic signal is warranted, the installation of a traffic signal would cost approximately \$117,000.

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